

PRELIMINARY

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MANUAL FOR WAVE RUNUP ANALYSIS
COASTAL FLOOD INSURANCE STUDIES

Prepared For

Federal Insurance Administration
Federal Emergency Management Agency

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Stone & Webster Engineering Corporation
Boston, Massachusetts

PRELIMINARY

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TABLE OF CONTENTS

<u>Section</u>	<u>Title</u>
1	INTRODUCTION
2	BASIC CONCEPTS
2.1	Maximim Runup Point
2.2	Wave Crest Profile
3	APPLICATION
4	METHODOLOGY
4.1	Data Acquisition
4.2	Data Reduction/Analyses
4.3	Wave Crest Profile Computations
5	SPECIAL SITUATIONS
6	EXAMPLE PROBLEM
6.1	Data Acquisition
6.2	Data Reduction/Analysis
6.3	Wave Crest Profile Computations
7	VERIFICATION
7.1	Laboratory and Field Observation
7.2	Historical Damage Information
8	REFERENCES AND BIBLIOGRAPHY
APPENDIX A WAVE RUNUP PROGRAM	
APPENDIX B DETERMINATION OF SIGNIFICANT WAVE HEIGHT AND PERIOD	

LIST OF FIGURES

<u>Figure</u>	<u>Title</u>
2-1	Wave Crest Profile
6-1	Segment of National Ocean Survey Chart
6-2	Segment of Work Map
6-3	Wave Envelope Plot
6-4	Breaker Height Index
7-1	Verification of Wave Crest Profile with Laboratory Data
7-2	Verification of Wave Crest Profiles with Damaged Data

LIST OF TABLES

<u>Table</u>	<u>Title</u>
4-1	Roughness Coefficient
4-2	Wave Length at Breaker Depth
6-1	Profile Data
6-2	Coastal Flood Levels, Southern Maine

SYMBOLS

- d_b - water depth at breaking (ft)
- d_s - maximum water depth of slope segment intersected by stillwater level
- H_b - breaking wave height (ft)
- H_i - various deepwater wave heights used for wave runup computations (ft)
- H_{min} - minimum wave height of H_i
- H_s or H'_o - deepwater significant wave height, or design wave height (ft)
- m - beach slope
- R_i - wave runups corresponding to H_i (ft)
- r - roughness coefficient
- T_s or T - significant wave period, or design wave period (sec)
- L_b - wave length at breaking (ft)

SECTION 1

INTRODUCTION

The coast of the northern Atlantic Ocean is subject to intense wave action during the hurricane and northeaster storm seasons. High astronomical tide and storm surge individually are not sufficient to inflict the types and degrees of damage observed on the coast. However, together these events combine to raise the stillwater level and allow accompanying waves to penetrate farther inland than permitted by normal water levels. Structures which in the absence of waves might experience little or no damage can be damaged when wave action is superimposed on elevated stillwater levels. The need exists to devise a method to account for this effect.

This manual describes a method developed by Stone & Webster Engineering Corporation (S&W) to determine wave crest elevations due to wave breaking and wave runup for use in Federal Emergency Management Agency (FEMA) coastal flood insurance studies. The scope of work (S&W 1979) calls for an analysis which will complement the National Academy of Sciences (NAS) wave height analysis method (NAS 1977, FEMA 1981). NAS's method had been developed for FEMA to analyze wave crest elevations seaward of the stillwater line. The wave height diminishes toward the stillwater line according to the NAS method. The S&W method determines wave crest elevations landward of the stillwater line and to a certain distance seaward of the stillwater line where it matches the NAS method. This manual has been prepared under FEMA contract No. H-4092.

The variation of wave height from initial breaking point to highest runup along a beach transect cannot be exactly specified. Existing coastal engineering literature, however, does provide a variety of graphs, reports, and laboratory and field measurements for breaking and broken wave crest analysis. The S&W method represents a simple and yet realistic wave crest profile consistent with existing state-of-the-art technology to describe wave height variation in the surf and runup zone. The references and bibliography section cites the results of a literature search initiated as part of the scope of work.

SECTION 2

BASIC CONCEPTS

Figure 2-1, Wave Crest Profile, presents a schematic of the wave crest envelope along a beach profile. The S&W method has been formulated to determine the maximum wave runup point and the wave profile between the runup point and NAS wave profile.

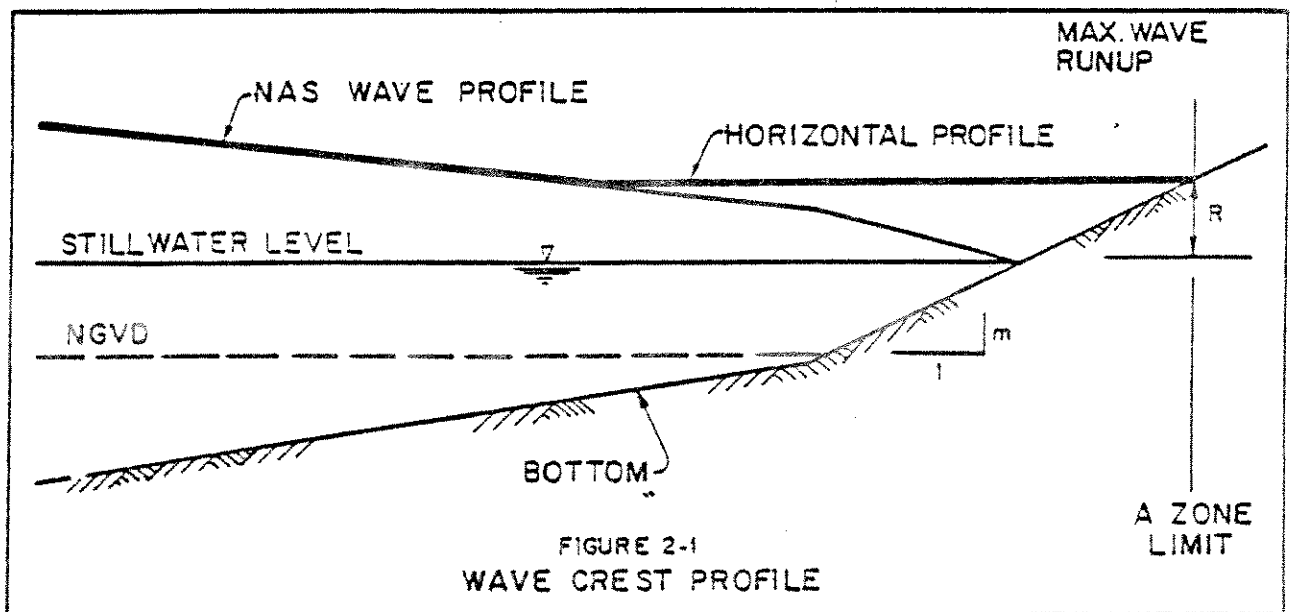
2.1 MAXIMUM RUNUP POINT

Runup curves (Stoa 1978b) are used in a computer program (Appendix A) to determine the maximum runup. Runup is a function of beach slope, m , deepwater wave height H_i , and wave period, T_s . H_i 's are a series of wave heights ranging from a minimum wave height, H_{\min} to the significant wave height, H_s for wave runup calculations. H_{\min} is a small wave height with a probability of 5 percent or less in a wave spectrum. For example, H_{\min} is 3 ft for 11-second period waves and 5 ft for 14-second period waves. In general, the maximum wave runup for a given wave spectrum is produced by waves of moderate height and long period (low wave steepness).

The wave runup computer program is operated with an ensemble of deepwater wave heights, H_i , a wave period, T_s , and the beach profile. The program yields corresponding wave runups, R_i , from which the maximum wave runup is obtained to define the point of maximum runup.

2.2 WAVE CREST PROFILE

The wave crest profile is constructed as follows. The profile based on the NAS methodology is drawn first. The NAS wave profile is characterized by having a wave crest which is 0.55 of its local depth above the stillwater level for open coastal areas without the presence of obstructions. A horizontal line parallel to the stillwater level is drawn from the maximum runup point toward offshore intersecting the NAS profile at a point that has the same elevation as the maximum runup point. This horizontal profile and the NAS profile seaward of the point of intersection constitute the wave crest profile along a beach transect.



SECTION 3
APPLICATION

The S&W methodology is applicable to all coastal areas subject to wind wave activities. The methodology was developed and verified for the southern Maine coastal areas which have relatively steep slopes. However, in many areas including some sections of New England coastline, the beach slopes are mild and the methodology need not be applied.

The calculated runup values on mild slopes are small and the ability of the runup to do damage is minimal. A criterion has been adopted which states that a runup value of less than 2 ft is incapable of causing significant damage. Based on the results of the Maine Flood Insurance Study, any slope less than 1 on 30 with a design wave period of 11 seconds or less, or any slope less than 1 on 35 with a design wave period of 14 seconds or less, produces less than 2 ft of runup. Since little damage occurs under these conditions, the runup calculation need not be applied when they are met. The slope cited above is defined as a straight line between two points on the ground profile; one point is 10 ft below and the other is 5 ft above the stillwater level. The slope criterion is given with the stipulation that the profile seaward of 10 ft below stillwater level is milder than the defined slope. If this is not the case the 2 ft runup slope criterion is not valid and the runup calculation should be performed.

SECTION 4
METHODOLOGY

The wave crest analysis methodology has three components, i.e., data acquisition, data reduction/analyses, and wave crest profile computations.

These three components are described below in chronological order. An example application is presented under Section 6.

4.1 DATA ACQUISITION

4.1.1 Beach Profiles

Beach profiles are required to determine breaking wave height, wave runup, and A and V zones. The user should select beach profile locations by field inspection and inspection of United States Geologic Survey (USGS) quadrangle sheets, National Ocean Survey (NOS) charts, aerial photography, and Flood Insurance Study work maps. During the inspection of these items, the user should select representative profiles for regions with similar coastal geomorphologic forms.

Coastal geomorphologic forms such as barrier beaches, headlands, tombolos, offshore islands, coves, bays, and bluffs can be identified. In addition, regions can be specified as having beach profiles of similar shape and slope. The wave crest elevations obtained from the methodology are sensitive to beach slopes and, as a consequence, care is required to assure that a representative beach profile has been selected for a given

region. Generally, profile spacing should be less than 1 mile.

After the user has selected beach profiles, a field trip is required to determine beach characteristics along the profiles. Characteristics such as sediment type and structure location should be noted.

Beach profiles will change seasonally. Summer beaches generally are mildly sloped and have wide berms, while winter beaches generally are steeply sloped and have narrow berms. A winter beach profile will have higher wave crest elevations than a summer beach profile for identical stillwater level and design wave conditions.

Since most hurricanes occur during the summer and most northeasters during the winter, a selection of two beach profile/design wave pairs might be needed; one for northeasters and one for hurricanes. In certain regions of the country, these data pairs are not needed since one storm type generally dominates, e.g., in New England northeasters predominate, thus the wave crest methodology requires winter beach profile data; in the South hurricanes predominate, thus the wave crest methodology requires summer beach profile data; while in the New York region, a mix of northeasters and hurricanes occurs, thus requiring both winter and summer beach profiles for the wave crest analysis.

Unfortunately, the profile data are generally limited to NOS charts seaward of mean low water, which presents beach profile data soundings on a given chart from numerous hydrographic surveys. These surveys might

have been performed at various times over the year. Therefore, it is not always possible to obtain typical winter/summer profiles. The user is advised to utilize NOS charts tied in with the FEMA work maps if no other profile data are available.

The beach profile data should be referenced to National Geodetic Vertical Datum (NGVD), which is mean sea level of 1929. The profiles should extend approximately from 40 ft below to 20 ft above stillwater level or to the highest elevation available.

Possible sources for beach profile data include:

- a. National Ocean Survey (NOS) Charts
- b. U.S. Geologic Survey (USGS) Quadrangle Sheets
- c. FEMA Work Maps
- d. Regional Coastal Information Center
- e. Others: U.S. Army Coastal Engineering Research Center, U.S. Army Corps of Engineers, State Offices of Coastal Zone Management, Local Universities, communities, and surveyors

4.1.2 Stillwater Level

The stillwater level is made up of astronomical tide and storm surge components. During the duration of a storm, the storm tide will vary as a

function of time. Generally, the storm tide will behave like a normal tide except that it will have higher high tide and higher low tide elevations.

The maximum water surface elevation is selected as the stillwater level for that storm. This selected stillwater level has an assigned frequency. The user may determine this level via mathematical model, frequency analysis, or literature search.

Possible data sources for stillwater level data include:

- a. ^{EMERGENCY MANAGEMENT AGENCIES} Federal Insurance Administration, FEMA
- b. U.S. Army Corps of Engineers, ^{COE}
- c. U.S. Army Coastal Engineering Research Center
- d. National Oceanic and Atmospheric Administration
- e. Regional Coastal Information Center
- f. State Agencies

4.1.3 Wave Height and Period

For wave runup calculations, wave heights and corresponding wave periods shall be obtained. The spectrum of waves at a given location is dependent on the available fetches, bottom topography, and prevailing storm type.

Possible wave data sources include:

- a. Environmental Data Service, NOAA
- b. National Climatic Center, NOAA
- c. U.S. Army Corps of Engineers
- d. U.S. Army Coastal Engineering Research Center
- e. National Oceanographic Data Center
- f. Regional Coastal Information Center

4.2 DATA REDUCTION/ANALYSES

4.2.1 Beach Profile

The beach profile data are plotted referenced to NGVD. Summer and winter profiles, if available or needed, should be plotted on separate profile paper. A sample beach profile data tabulation sheet and a beach profile figure are presented under Section 6, Example Problem.

4.2.2 Stillwater Level

The 100-year stillwater storm tide elevation is used for determining A and V zones, while the 10-year stillwater elevation is required for numbering the A and V zones. The user should determine the variation of stillwater level along the study area. The stillwater levels are plotted with the beach profile.

4.2.3 Wave Height and Period

The range of wave heights which should be used for runup calculations in order to determine the maximum runup is generally between a minimum of 3 ft to the significant wave height, H_s , based on the wave spectrum information at a given location. The significant wave period, T_s , may be used as the corresponding wave period for all wave heights in the wave runup calculation.

4.3 WAVE CREST PROFILE COMPUTATIONS

4.3.1 Beach Profiles

The beach profile is generated by plotting ground elevations versus horizontal distances. Beach slopes must be calculated between ground elevation points and a roughness coefficient must be supplied. Typical roughness coefficients are given on Table 4-1. A roughness coefficient of 1 is suggested when no direct beach observations are available.

In addition, for each slope segment the user should compute a breaker height, H_b , for each wave height, H_i , and wave period, T_s . Figure 7-3 in the Shore Protection Manual should be used to compute H_b and is reproduced in Section 6 as Figure 6-4. This information is to be used for runup calculations.

TABLE 4-1

ROUGHNESS COEFFICIENTS
(SPM after Battjes 1974)

<u>Slope Surface Characteristics</u>	<u>Placement</u>	<u>Roughness Coef. r</u>
Smooth Impermeable	_____	1.00
Concrete Blocks	Fitted	0.90
Basalt Blocks	Fitted	0.85-0.90
Gobi Blocks	Fitted	0.85-0.90
Grass	_____	0.85-0.90
One Layer Quarrystone (Impermeable Foundation)	Random	0.80
Quarrystone	Fitted	0.75-0.80
Rounded Quarrystone	Random	0.60-0.65
Three Layers of Quarry- stone (Impermeable Foundation)	Random	0.60-0.65
Quarrystone	Random	0.50-0.55
Concrete Armor Units (50% Void Ratio)	Random	0.45-0.5

4.3.2 Wave Crest Elevations

The maximum runup point is obtained from operating the wave runup computer program. The program incorporates laboratory runup data (Stoa 1978b) and uses the composite beach slope method developed by Saville (1958). An ensemble of deepwater wave heights, H_i and wave period, T_s , is used as input to the program. The wave runup program calculates an array of wave runups, R_i , corresponding to the input wave characteristics H_i , and T_s . The user selects the maximum computed runup from the values R_i and defines the maximum runup point, R_{max} .

A detailed description of the program and operating instructions are presented in Appendix A - Wave Runup Program.

In addition, the R_{max} will require correction if a beach berm is present along the profile. The correction is given by:

$$\frac{R_{corrected}}{R_{max}} = 1 + \left(\frac{\text{Berm width}}{L_b} \right)^{3/4}$$

where L_b (wave length at breaking) can be obtained from Table 4-2, Wave Length at Breaker Depth, via log/log interpolation (Dean 1974).

TABLE 4-2

WAVE LENGTH AT BREAKER DEPTH

<u>d_b/L_o</u>	<u>L_b/L_o</u>	<u>Notes:</u>
0.02	0.422461	L_b - wave length at breaking
0.05	0.627344	L_o - deepwater wave ₂ length = $5.12 T^2$
0.10	0.824424	d_b - breaker depth

Refer to the references of NAS 1977 and FEMA 1981 for the delineation of the NAS wave crest profile.

SECTION 5

SPECIAL SITUATIONS

The method described above has limitations for certain beach and structural sections. Special attention is needed when the following situations are encountered:

1. Convex Profile Runup Oscillation - For a convex beach profile, the runup computed by the composite slope method will oscillate between two runup values. When this occurs, the runup program prints the two runup values from the last two iterations. Engineering judgement should be exercised to select a reasonable value between these two runups.
2. Wave Overtops Profile - If the runup value overtops the profile, consult Shore Protection Manual and Coastal Engineering Technical Aid (CETA) 77-7 for methodology to estimate overtopping flow rate. A special problem report should be issued indicating the site-specific methodology to estimate overtop flooding.
3. Stillwater Level Intersects Vertical Sea Wall - User should consult Shore Protection Manual and CERC TP-78-2 for analysis of vertical runup. If this analysis shows that overtopping occurs, the user should use CETA 77-7, cited above, for determining overtopping flow rate and issue a Special Problem Report indicating the site-specific methodology used to estimate overtop flooding.

SECTION 6

EXAMPLE PROBLEM

The problem presents a wave crest analysis for a transect located in Old Orchard Beach for the coastal flood insurance study in Maine.

6.1 DATA ACQUISITION

6.1.1 Beach Profiles

The NOS Chart and Flood Insurance Work Maps needed for this analysis were obtained. Figures 6-1 and 6-2 present segments of these maps and charts. The transect of interest (Transect 71) is marked on both maps. Notice that the Maine State Coordinate Grid System is drawn on both figures. This is to ensure the transect is accurately presented on the two figures. The transect elevation data are presented on Table 6-1, Profile Data.

6.1.2 Stillwater Level

Storm tide elevations (stillwater levels) for the 10-, 50-, 100-, and 500-year events were determined for nine coastal communities in York County, Maine. The major tasks included the simulation of wind and pressure fields of northeasters and hurricanes, the determination of coastal storm surge and storm tide elevations, and the statistical analysis of storm tide elevations (S&W 1978a).

TABLE 6-1

PROFILE DATA
(TRANSECT 71, MAINE FLOOD INSURANCE STUDY)

Elevation (ft) relative to		Distance (ft) from waterline		Slope m	Roughness Coefficient r	Profile Section Number
MLW	NGVD	MHW	NGVD			
-18	-22.3	2460	-2420	.005	1.0	1
-12	-16.3	1170	-1130	.011	1.0	2
-6	-10.3	630	-590	.014	1.0	3
(MLW) 0	-4.3	210	-170	.025	1.0	4
(NGVD) 4.3	0.	38	0			
(MHW) 9.0	+ 4.7	0		.125	1.0	5
	+ 5.	2	40	.063	1.0	6
	+ 10.	82	120	.028	1.0	7
	+ 15.	262	300			

NGVD - MLW = 4.3 ft.
MHW - NGVD = 4.7 ft.

Mean Tide Range = 9.0 ft.

TABLE 6-2

COASTAL FLOOD LEVELS MEASURED FROM NGVD
 IN THE STUDY AREA
 (S&W, 1978a)

<u>Location</u>	Coastal Flood Stillwater Level Measured from NGVD at Given Return Intervals (in feet)			
	<u>10-Year</u>	<u>50-Year</u>	<u>100-Year</u>	<u>500-Year</u>
Portland	8.51	9.15	9.41	10.01
Cape Elizabeth	8.28	8.93	9.19	9.79
<u>Old Orchard Beach</u>	<u>8.21</u>	8.83	<u>9.09</u>	9.67
Sacco Biddeford	8.09	8.73	8.99	9.59
Kennebunkport Kennebunk	8.22	8.82	9.07	9.63
Wells	7.98	8.56	8.79	9.31
Ogunquit	7.95	8.52	8.75	9.27
York	7.85	8.42	8.65	9.17
Kittery Portsmouth	7.86	8.45	8.68	9.21

The 100-year and 10-year elevations for Transect 71, which is located in the town of Old Orchard Beach, are, respectively, 9.1 ft and 8.2 ft (see Table 6-2).

6.1.3 Wave Height and Period

The significant wave height and period in this study area are determined to be 30 ft and 14 seconds, respectively. See Appendix B for details of significant wave height and period determinations. The range of wave heights, H_i , to be used for wave runup calculation is, therefore, from 5 to 30 ft. The associated wave period is 14 seconds for all wave heights.

6.2 DATA REDUCTION/ANALYSIS

6.2.1 Beach Profile

Figure 6-3 presents the beach profile data plotted on section paper.

6.2.2 Stillwater Level

The 100-year and 10-year stillwater levels are indicated on Figure 6-3.

6.3 WAVE CREST PROFILE COMPUTATIONS

6.3.1 Maximum Runup Point

Maximum runup values are obtained from the Wave Runup Computer Program.

Wave runups values are computed for each of the deepwater (unbroken) wave heights of 5, 7, 9, 11, and 13 ft with 14 second wave period for both 10- and 100- year storm tide levels. For the 10-year level a maximum runup value of 3.9 ft resulted from a deepwater wave height of 5 ft. The maximum runup of 3.7 ft for the 100-year level resulted from a 7 ft deepwater wave height. When added to the stillwater levels these runup values yield 12.1 ft and 12.8 ft NGVD runup elevations respectively for the 10- and 100-year storm levels.

The following breaker heights for those profile sections under the stillwater level and five deepwater wave heights are used as input to the Wave Runup Computer Program (see A.5.2, Appendix A).

Profile	Breaker Height, H_b						
Section	for $T = 14$ seconds						
No.	m	H_o (ft):	5	7	9	11	13
1	.005		7.2	9.7	11.3	13.4	15.1
2	.011		8.0	10.5	12.5	14.7	16.6
3	.014		8.3	10.8	12.8	15.0	17.0
4	.025		8.9	11.5*	13.7	16.0	18.0
5	.125		10.8	13.8	16.4	19.0	21.3
6	.063		10.0	12.8	15.2	17.9	20.0

* $H_o/gT^2 = 7/(32.2 \times 196) = 0.0011$. From Figure 6-4, $H_b/H_o = 1.64$ is obtained.

Hence, $H_b = 1.64 \times 7 = 11.5$

6.3.2 Wave Crest Profile

The NAS profile having a wave crest above the stillwater level of 0.55 of its local depth is calculated and plotted on the transect (Figure 6-3). The NAS wave crest profile approaches the stillwater level at the point where the stillwater intersects the ground profile. A horizontal line parallel to the stillwater level is drawn from the maximum runup point, 12.8 ft NGVD for the 100-year flood and 12.1 ft NGVD for the 10-year flood, toward offshore intersecting the NAS profile. The combination of the horizontal line and the NAS profile offshore beyond the point of intersection constitutes the final wave crest profile.

A and V zone determinations are made. The transect has been drawn previously on the work maps; and A and V zones are then transferred from the figures to the work map.

(Saco Bay and Vicinity)

SOUNDINGS IN FEET - SCALE 1:20,000

13287

(formerly C&GS 231)

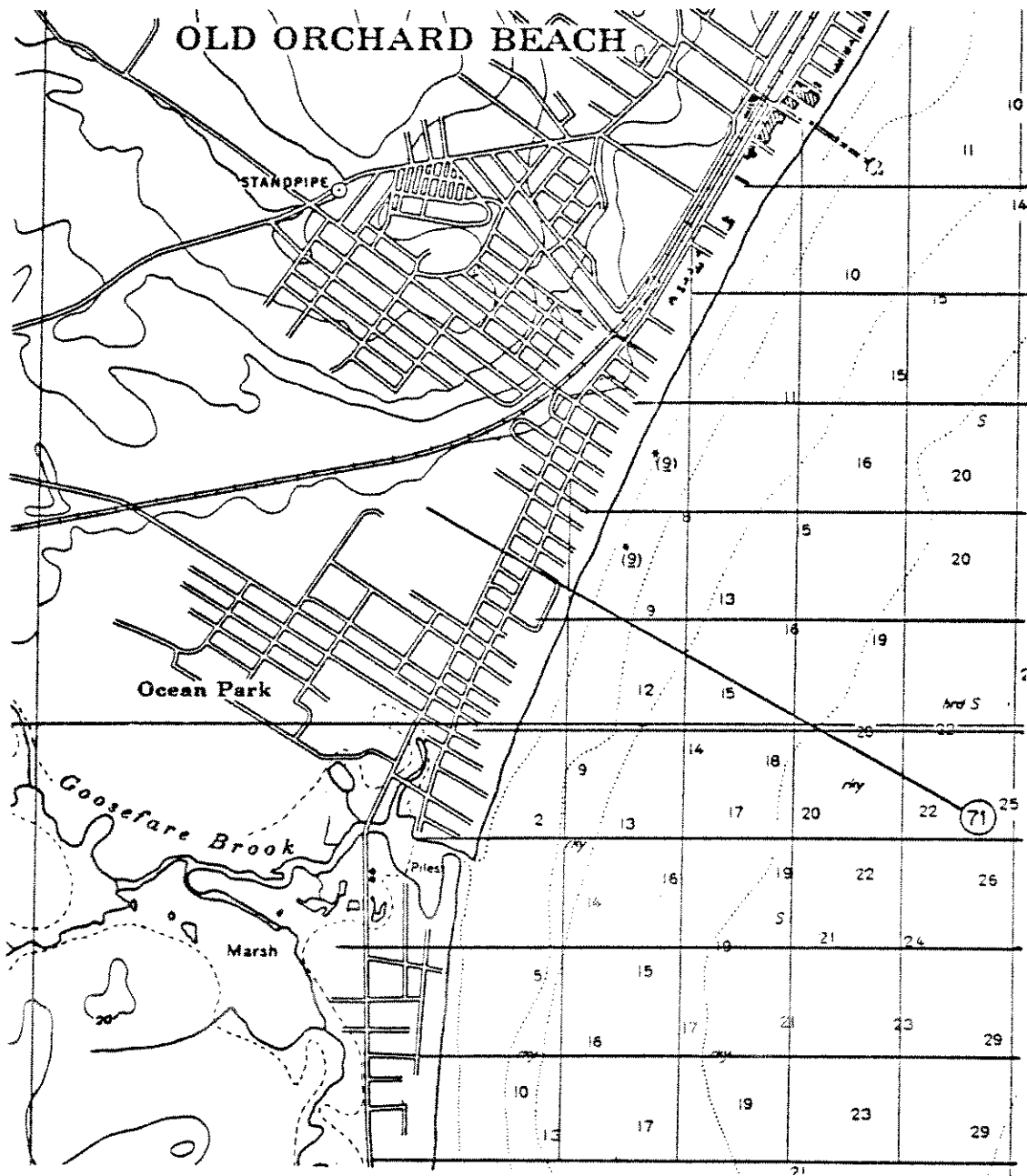


FIGURE 6-1
SEGMENT OF N.O.S. CHART
FEDERAL INSURANCE ADMINISTRATION
MANUAL FOR WAVE RUNUP ANALYSIS
COASTAL FLOOD INSURANCE STUDIES
STONE & WEBSTER ENGINEERING CORPORATION

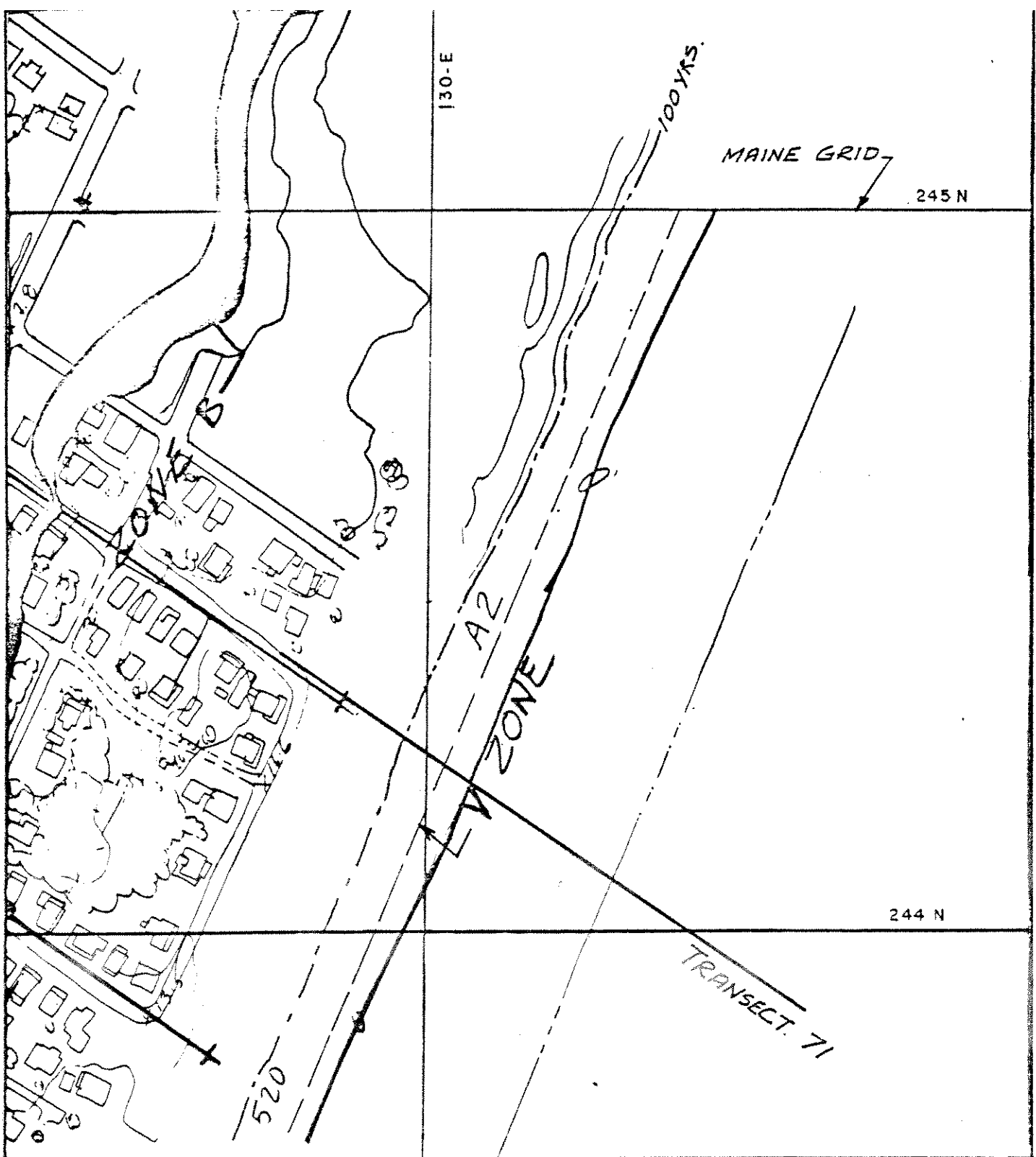
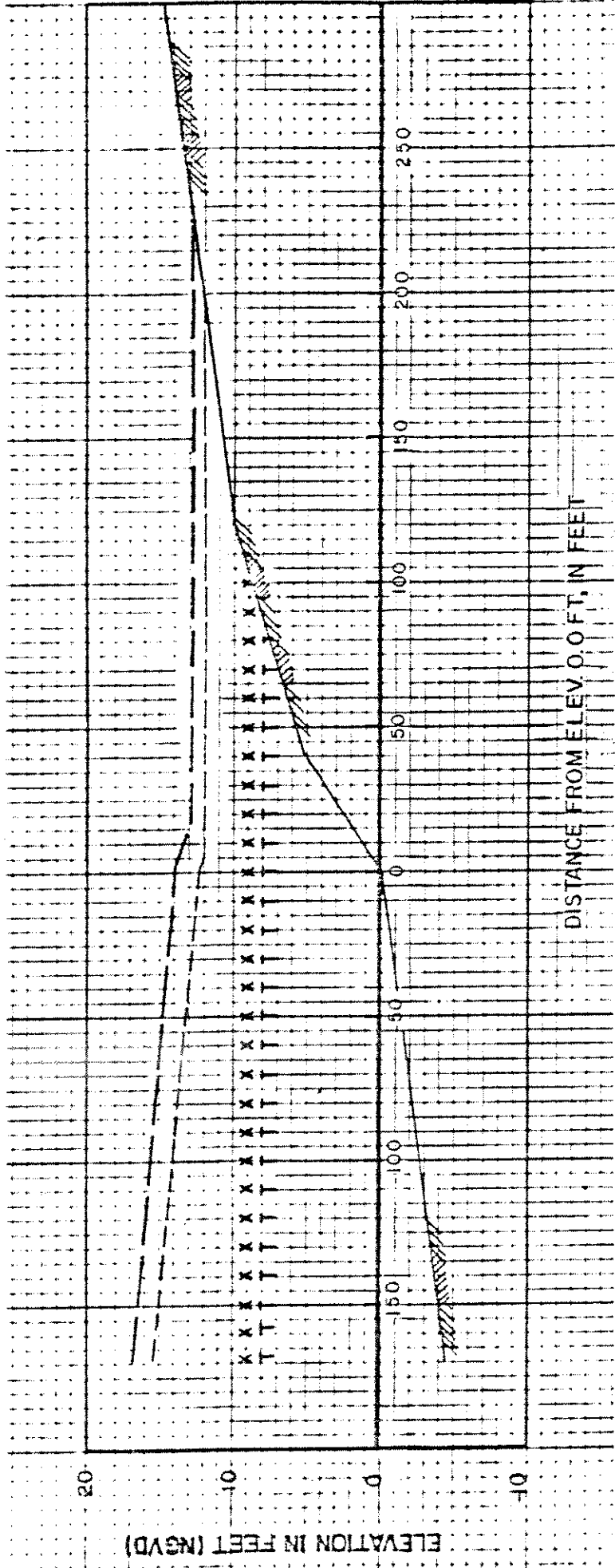


FIGURE 6-2
SEGMENT OF WORK MAP
FEDERAL INSURANCE ADMINISTRATION
MANUAL FOR WAVE RUNUP ANALYSIS
COASTAL FLOOD INSURANCE STUDIES
STONE & WEBSTER ENGINEERING CORPORATION

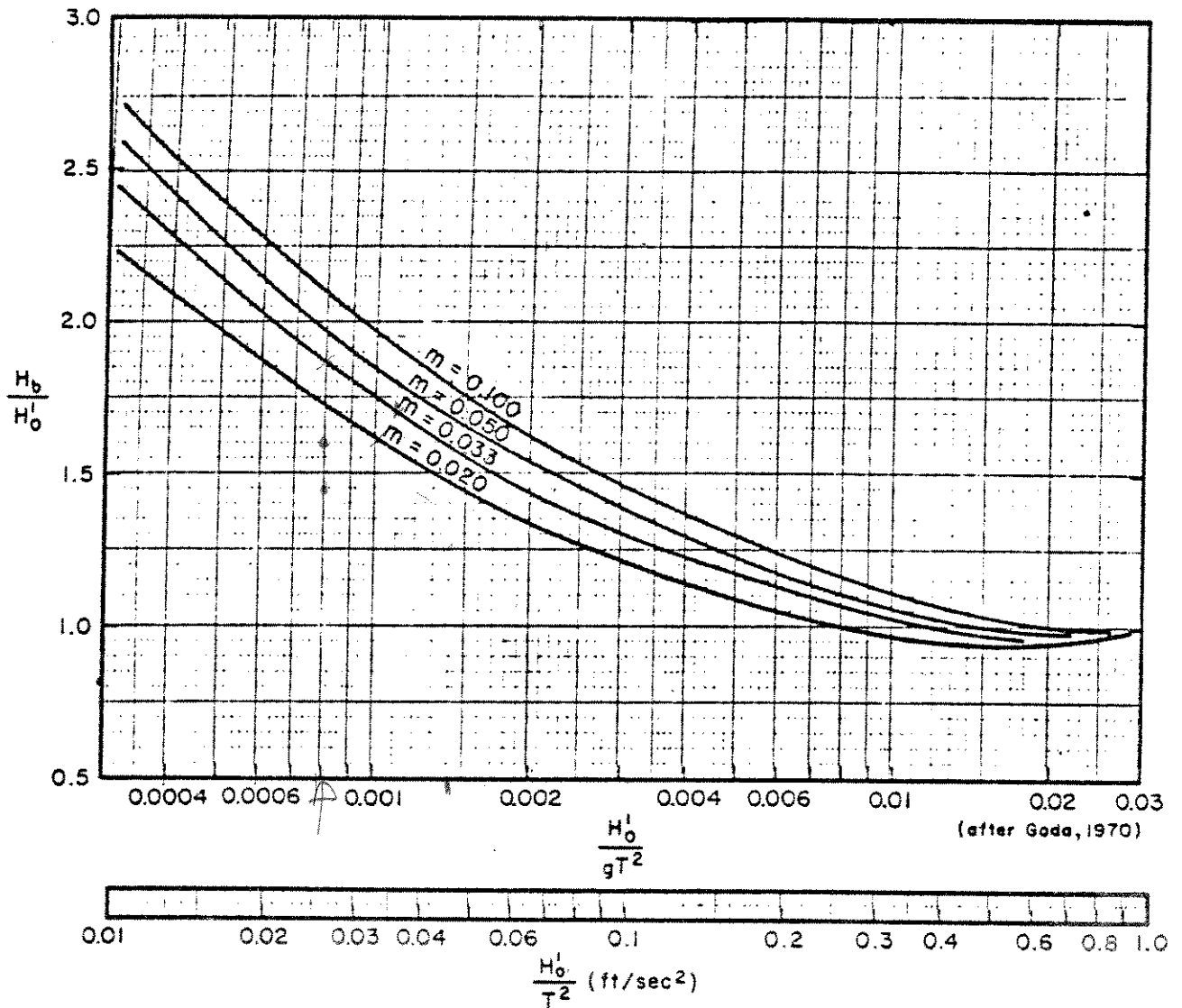


LEGEND

- 100 YEAR WAVE ENVELOPE
- X X X X X X 100 YEAR STORM SURGE ELEVATION
- - - 10 YEAR WAVE ENVELOPE
- TTTTTTTTTT 10 YEAR STORM SURGE ELEVATION
- ////// GROUND PROFILE

WAVE ENVELOPES
TRANSECT 71, OLD ORCHARD BEACH

FEDERAL EMERGENCY MANAGEMENT AGENCY
Federal Insurance Administration
TOWN OF OLD ORCHARD BEACH, ME
(YORK CO.)



Breaker Height Index, H_b/H_0 Versus Deep Water
Wave Steepness, H_0^1/gT^2

(SHORE PROTECTION MANUAL, FIG. 7-3)

FIGURE 6-4
BREAKER HEIGHT INDEX
FEDERAL INSURANCE ADMINISTRATION
MANUAL FOR WAVE RUNUP ANALYSIS
COASTAL FLOOD INSURANCE STUDIES
STONE & WEBSTER ENGINEERING CORPORATION

SECTION 7

VERIFICATION

The wave crest profile, as determined by the method outlined in this report, has been verified with laboratory observations and actual wave damage information.

7.1 LABORATORY AND FIELD OBSERVATIONS

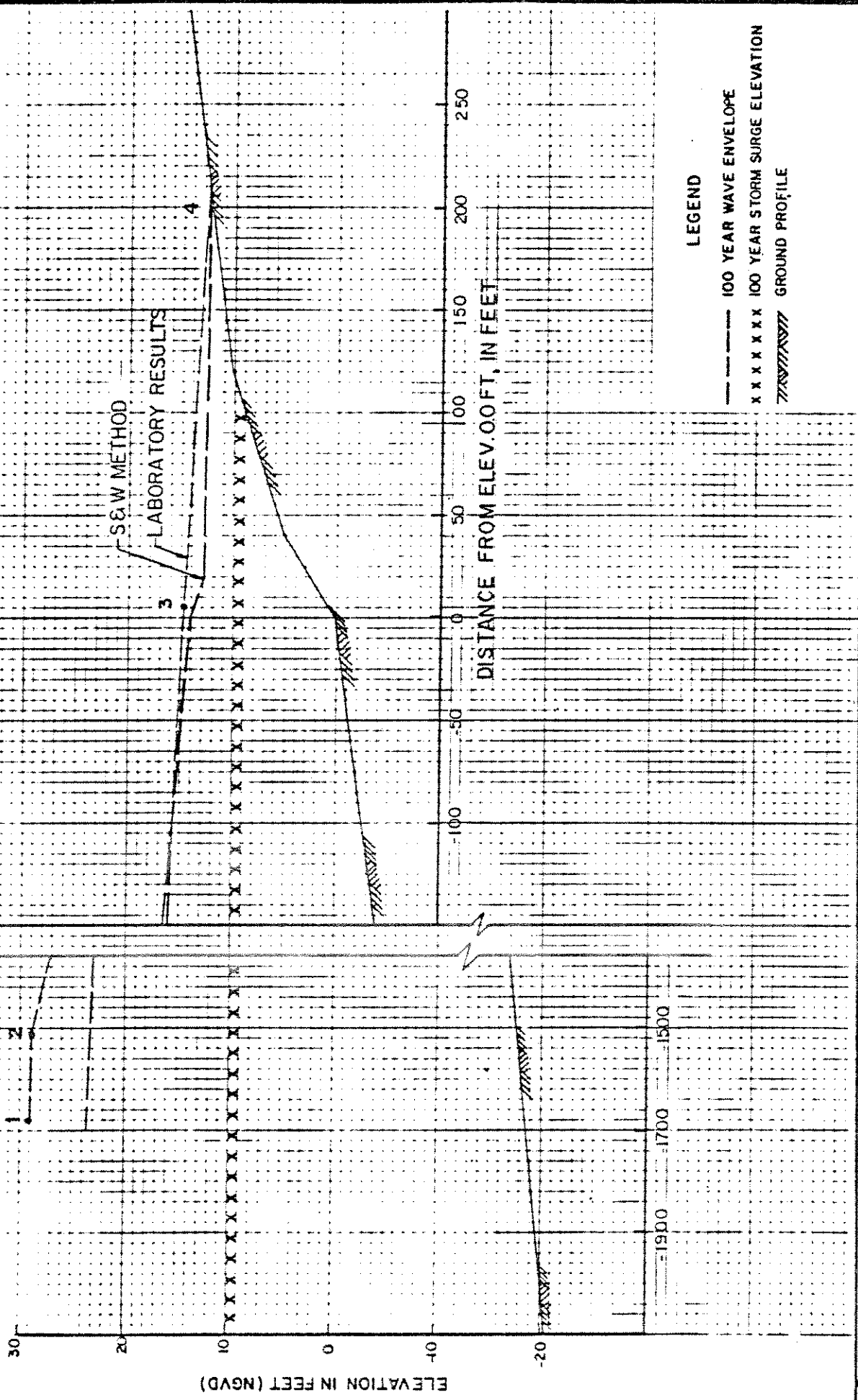
Deepwater waves are formed offshore of coastal areas due to storm activities. These storm waves have a significant wave height, H_s , and period, T_s , which represent the spectrum of wave heights and periods generated by the storm.

As the significant wave propagates toward shore, shoaling and refraction occur. Eventually it reaches a depth of water where it breaks. At this depth, d_b , the original wave height, H_s , is transformed into a breaker height, H_b , with 90 percent of the wave height above the stillwater level (U.S. CERC Shore Protection Manual, 1977). The breaking wave travels up to a distance without reducing its crest elevation and then decays to a residual wave height near the shore in a manner which can best be described by laboratory test results. The residual wave height near the shore is defined from laboratory work (Horikawa and Kuo, 1966, Nakamura et al, 1966, and Van Dorn, 1978) as occurring at a water depth of $0.3 d_b$ with a wave crest elevation of $0.25 H_b$ above the stillwater level.

The wave crest profile based on the above scenarios connecting the maximum wave runup point is compared with the S&W profile for the sample problem transect. As shown in Figure 7-1, the two 100-year wave profiles are in good agreement.

7.2 HISTORICAL DAMAGE INFORMATION

More than twenty (20) transects were taken across the reported damage areas for verification purposes. Most of the damage which occurred during the February 1978 storm in southern Maine coastal areas was above the storm tide stillwater level. The wave damaged areas are identified on the predicted 100-year wave envelope as shown in Figure 7-2 which is typical of the damaged transects. As can be seen from the figure the maximum wave runup points are above the reported damage locations.

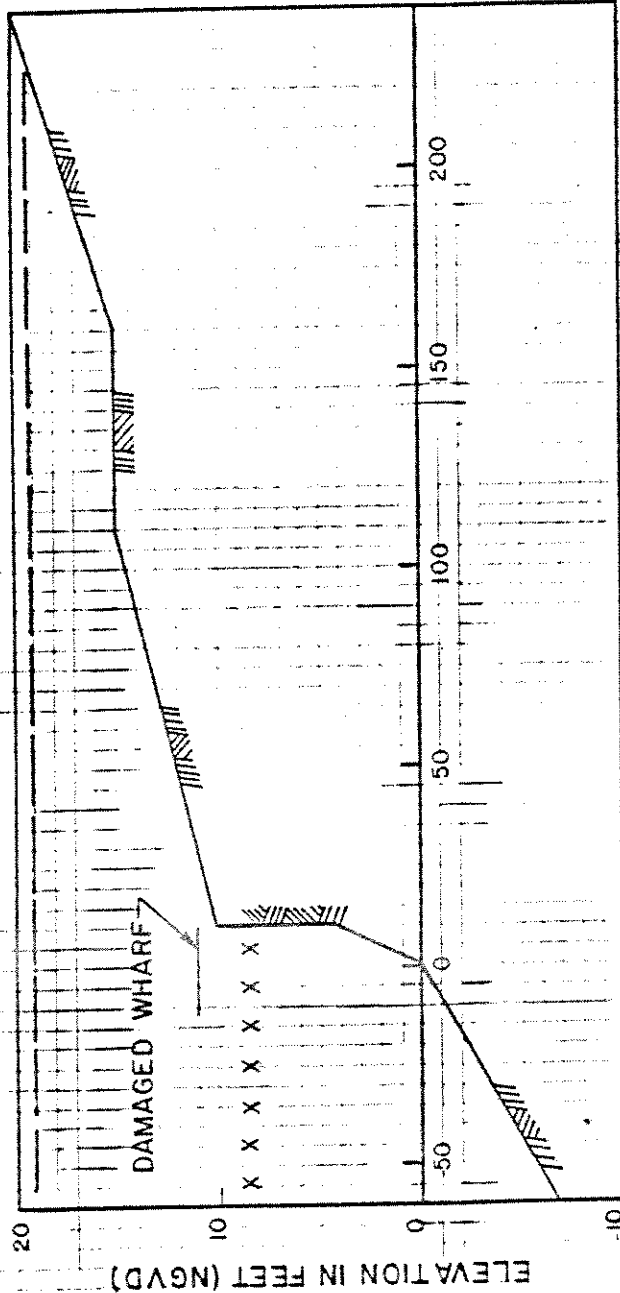


FEDERAL EMERGENCY MANAGEMENT AGENCY
 Federal Insurance Administration

WAVE ENVELOPES

TRANSECT 71

FIG. 7-1



LEGEND
 --- 100 YEAR WAVE ENVELOPE
 X X X X X 100 YEAR STORM SURGE ELEVATION
 // // // // // GROUND PROFILE

DISTANCE FROM ELEV. 0 FT. IN FEET

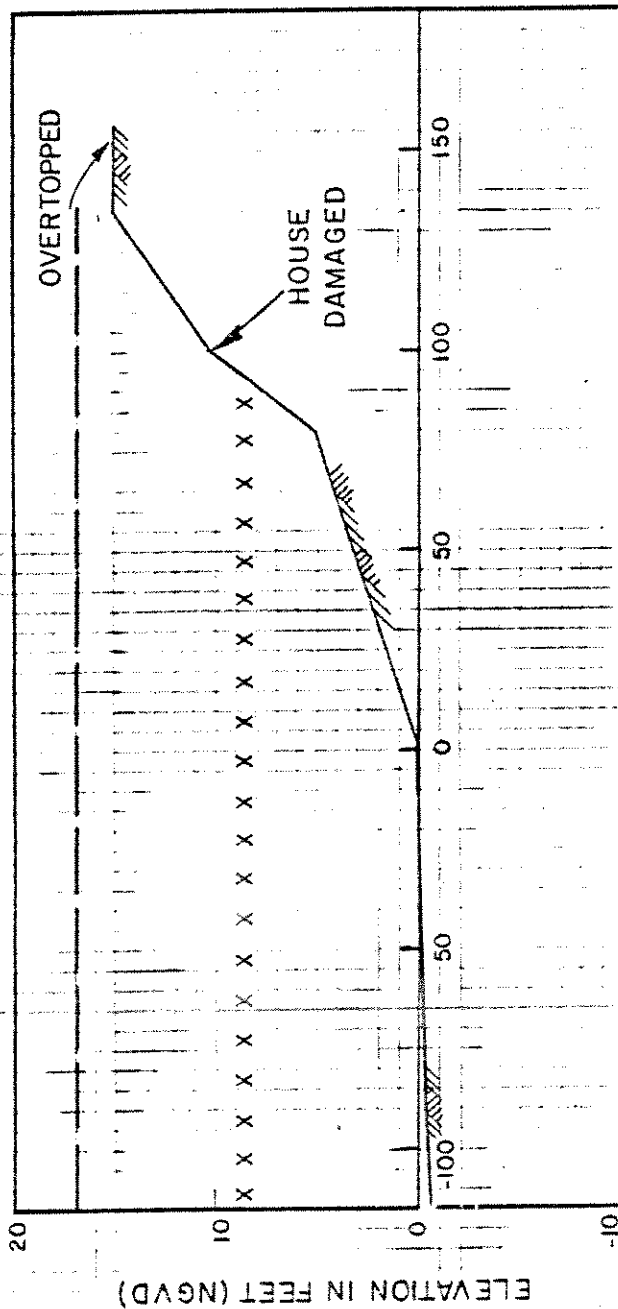
DAMAGE INFORMATION: FRISBEE WHARF SUSTAINED DAMAGE TO PILING, RAMPS AND DECKING. THE DECKING IS AT APPROX. 10.9' NGVD.

WAVE ENVELOPES

TRANSECT 1, KITTERY

FEDERAL EMERGENCY MANAGEMENT AGENCY
 Federal Insurance Administration

TOWN OF KITTERY, ME
 (YORK CO.)



DAMAGE INFORMATION: HOUSE ON THE
 CAPE NEDDICK COVE WITH ITS BASE
 AT THE 10 FT NGVD LEVEL WAS
 DAMAGED

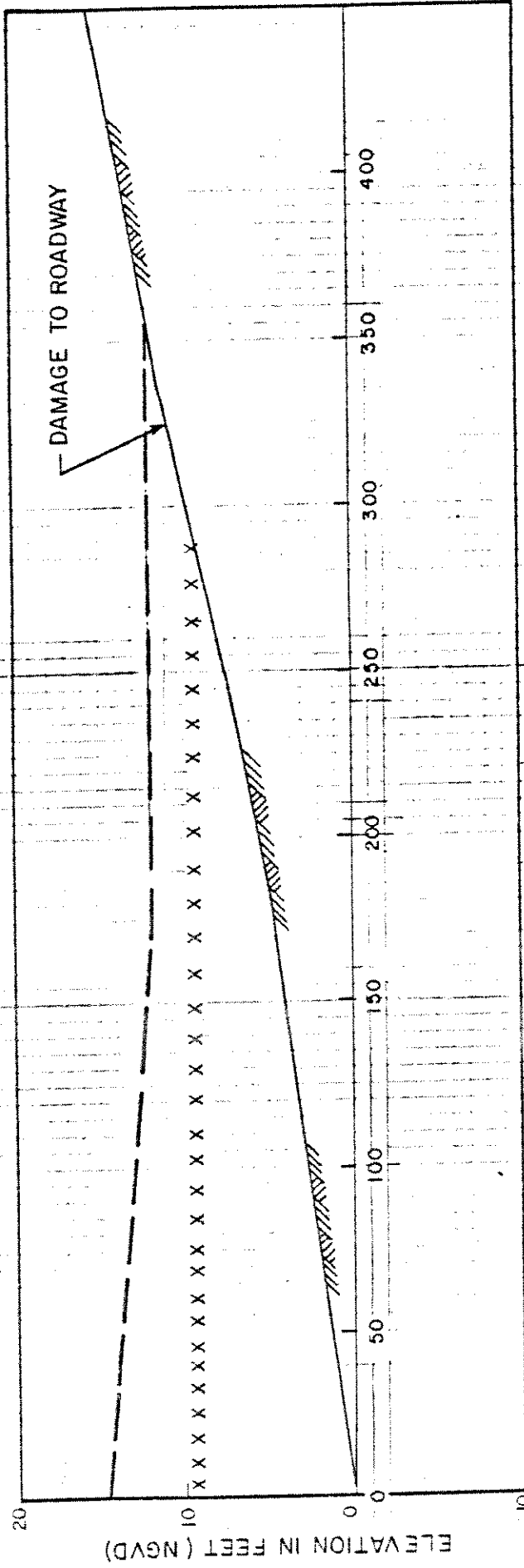
LEGEND
 --- 100 YEAR WAVE ENVELOPE
 X X X X X 100 YEAR STORM SURGE
 ELEVATION
 [Hatched] GROUND PROFILE

DISTANCE FROM ELEV. 0.0 FT., IN FEET

FEDERAL EMERGENCY MANAGEMENT AGENCY
 Federal Insurance Administration
 TOWN OF YORK, ME.
 (YORK CO.)

WAVE ENVELOPES

TRANSECT 19-1, YORK



LEGEND

- 100 YEAR WAVE ENVELOPE
- X X X X 100 YEAR STORM SURGE ELEVATION
- ////// GROUND PROFILE

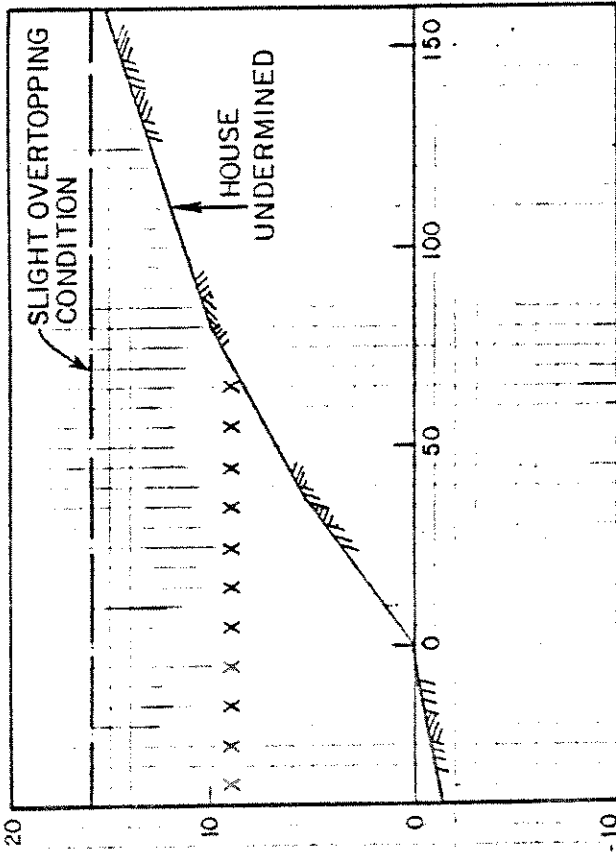
DISTANCE FROM ELEV.O.O.F.T, IN FEET

DAMAGE INFORMATION: A ROAD ALONG THIS BEACH WAS DAMAGED DURING THE 1978 BLIZZARD. THE LOWEST ELEVATION OF THIS ROAD IS ~10.5' NGVD.

FEDERAL EMERGENCY MANAGEMENT AGENCY
 Federal Insurance Administration
 TOWN OF BIDDEFORD, ME
 (YORK CO.)

WAVE ENVELOPES

TRANSECT 62, BIDDEFORD



DAMAGE INFORMATION: IN THE CAMP ELLIS AREA A HOUSE AT ~12' NGVD WAS UNDERMINED AND TIPPED DURING THE 1978 BLIZZARD.

LEGEND
 --- 100 YEAR WAVE ENVELOPE
 X X X X X 100 YEAR STORM SURGE ELEVATION
 [Hatched Area] GROUND PROFILE

FEDERAL EMERGENCY MANAGEMENT AGENCY
 Federal Insurance Administration

TOWN OF SACO, ME
 (YORK CO.)

WAVE ENVELOPES

TRANSECT 68-1, SACO

SECTION 8

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APPENDIX A: WAVE RUNUP PROGRAM USER'S MANUAL

- A.1 Program Summary
- A.2 Computer Requirements
- A.3 Functional Description
 - A.3.1 Technical Algorithm
 - A.3.2 Block Diagram
 - A.3.3 Program Description
- A.4 Operating Instructions
 - A.4.1 Program Input
 - A.4.2 Program Output
 - A.4.3 Error Messages
 - A.4.4 Sample IBM Job Control Language
- A.5 Sample Problem
 - A.5.1 Problem Description
 - A.5.2 Sample Input
 - A.5.3 Sample Output
- A.6 Program Listing

APPENDIX A

Wave Runup Program

A.1 Program Summary

Purpose: To calculate the wave runup on both single and composite slopes.

Scope: The program takes input values of the beach profile and wave characteristics to compute wave runup above the stillwater level. The computer program's data base is a discretization of Stoa's (1978b) runup curves which are derived from laboratory experiments. The program uses a single slope technique for waves that do not runup past the first slope landward of the stillwater level. The principle of Saville's (1958) Composite Slope Method is used for waves that do runup past the first landward slope. In addition, the program adjusts the computed runup for laboratory model to prototype scale differences and for roughness along the profile surface.

Input:

1. Profile points that define the shape of the beach or structure
2. Roughness correction factors for each slope section between profile points

3. Stillwater level
4. Deepwater wave height
5. Wave period
6. A breaking wave height for each slope section below the stillwater level

- Output:
1. Wave and water level input parameters
 2. Cotangent of each slope section
 3. Slope sequence number that identifies which slope section the wave breaks on
 4. Slope sequence number that identifies which slope section the runup limit reaches
 5. The runup above the stillwater level

A.2 Computer Requirements

The wave runup program was developed on an IBM 370/3033, in the FORTRAN IV programming language. The IBM FORTRAN H-EXTENDED compiler was selected for the program development and for production runs.

Compilation requires approximately 5 seconds of computer time. Computer execution time requires approximately 0.2 seconds for one profile, but varies according to the number of wave conditions input and the number of iterations required for convergence. Core requirements are in the 250-300K range.

A.3 Functional Description

A.3.1 Technical Algorithm

Wave runup on a composite slope is determined by using laboratory derived curves for single slopes. The method is one of successive approximations, involving the replacement of the actual composite profile with a hypothetical single slope extending from the wave breaking point to the previously estimated runup value. A final runup is found when two successive iterations yield the same runup value.

The program input parameters are discretized (depth and length) points describing the profile, the deepwater wave height and period, and the roughness factor and breaking wave height for each slope section. The breaking wave height, H_b , is found by entering Figure 7-3 in the Shore Protection Manual (1977) with beach slope and deepwater wave steepness. The first task performed by the computer program is to find the depth, d_b , at which the incoming wave breaks. The following iterative procedure determines d_b :

1. Start at the most seaward slope section on the composite profile.
2. Calculate the breaking depth from the following Shore Protection Manual (1977) equations (2-91, 2-92, 2-93);

$$d_b = \frac{H_b}{b - (a H_b / gT^2)} \quad (A-1)$$

where, $a = 43.75 (1 - e^{-19m})$, (A-2)

$$b = \frac{1.56}{(1 + e^{-19.5m})} \quad (A-3)$$

m is the slope for the given slope section and T is the wave period.

3. Check to see if the computed breaking depth, d_b , can physically exist on the current slope section given the stillwater depth above the section. If the computed d_b is not large enough, consider the next slope section landward and repeat step 2.

The proper breaking depth is found when it can exist in the water over the given slope section. If the breaking depth for a slope section is deeper than any point on the slope section, then the wave is assumed to break at the point between the current slope section and the section immediately seaward.

Once the breaking depth is found, an iterative algorithm to find the runup, R , is started. The ratio of structure depth to deep-water wave height, d_s/H_o' , and the slope of the profile section immediately seaward of d_s (slope section 3 in Figure A-1) are the parameters used to determine which family of curves is used and H_o'/T^2 is computed to determine which curves within that family are used. A family of curves is defined as being one of Stoa's

(1978b) runup charts. One family is unique from other families of curves because of the structure approach slope and the d_s/H_0' ratio.

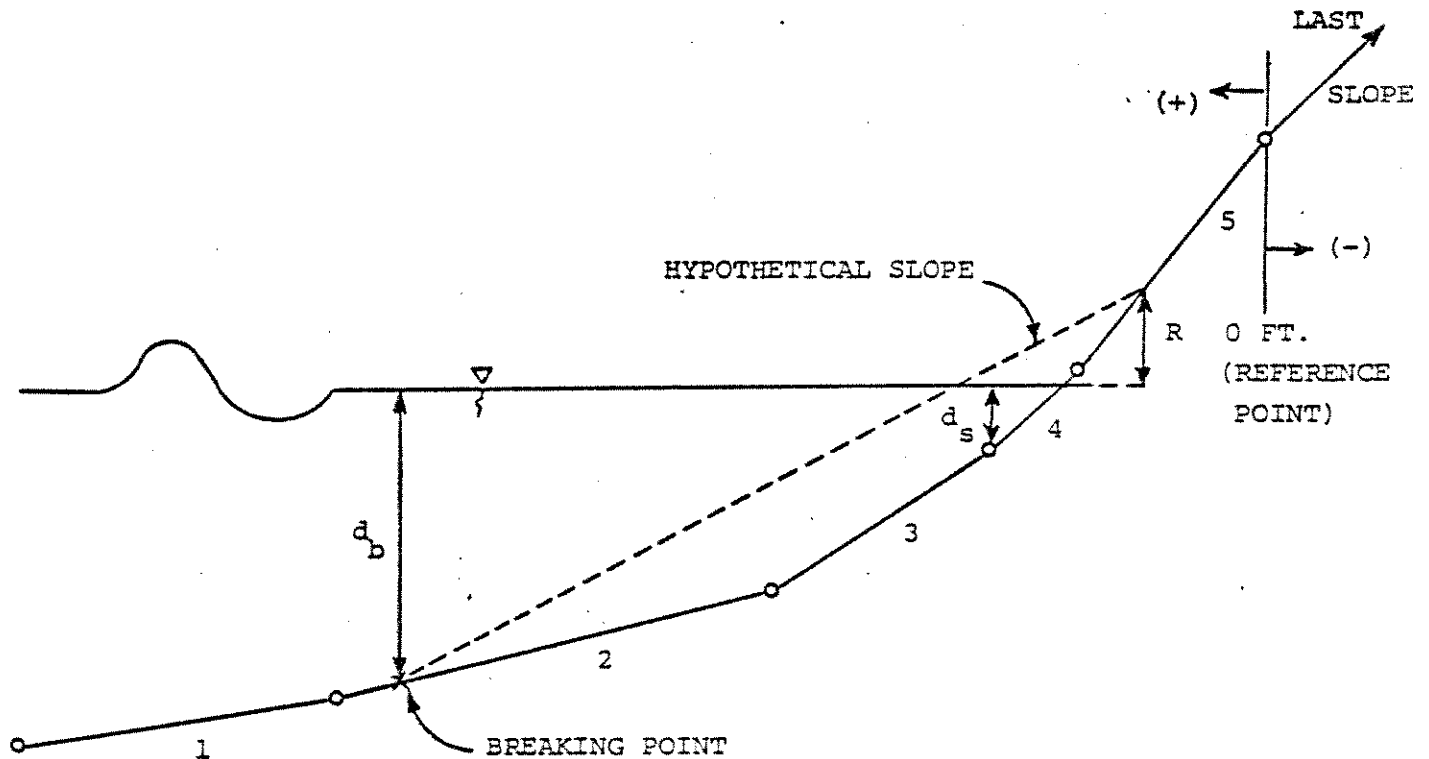


Figure A-1. Typical Composite Profile

Once the correct curves are found, the slope of the profile section where the water level intersects (slope section 4 in Figure A-1) is entered into the curves to find the surrounding R/H_0' curve values. Interpolations between the curves yields a specific R/H_0' value. Multiply this ratio by H_0' to find the runup, R , above the stillwater line. If R does not exceed the slope section that the water level intersects (slope section 4), then

this computed runup is taken as the final answer. If the runup does exceed slope section 4, then the program enters the composite slope algorithm. In the composite slope method, the parameter d_s/H_o' is replaced by d_b/H_o' (Saville, 1958). The ranges for both d_b/H_o' and d_s/H_o' that indicate which family of curves to use are in Table A-1. Families of curves 1-7 are in order, Figures 8, 9, 10, 11, 2, 3, and 4 from Stoa (1978b).

TABLE A-1

<u>Family of Curves</u>	<u>Approach Slope (cotθ)</u>	<u>Range of d_s/H_o' or d_b/H_o'</u>
1	ANY	$0 \leq d/H_o' \leq .8$
2	ANY	$0.8 < d/H_o' \leq 1.3$
3	ANY	$1.3 < d/H_o' \leq 2.5$
4	≤ 30	$2.5 < d/H_o'$
5	> 30	$2.5 < d/H_o' \leq 4.0$
6	> 30	$4.0 < d/H_o' \leq 6.5$
7	> 30	$6.5 < d/H_o'$

The program procedure from the second iteration on is as follows:

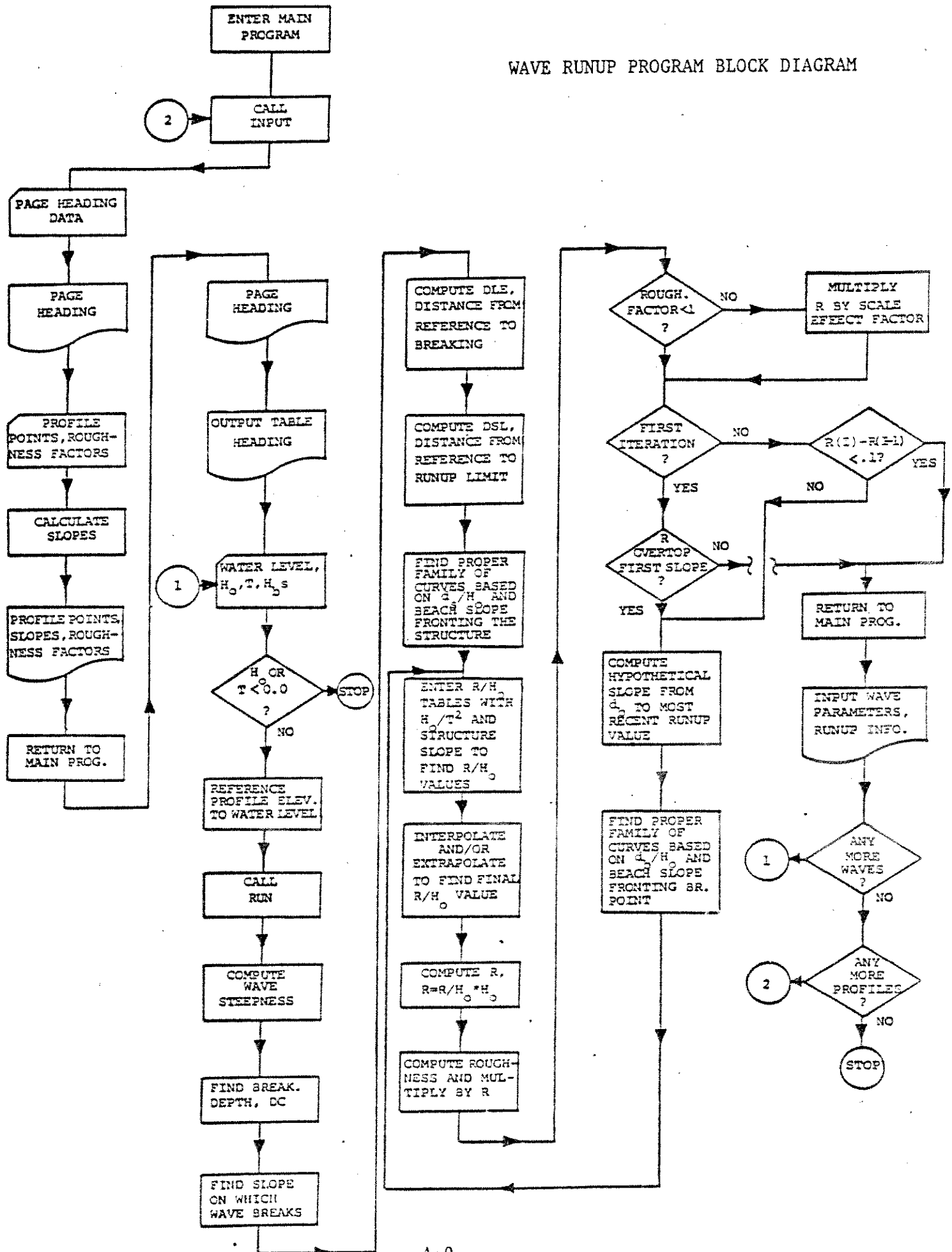
1. Determine which family of curves will be entered based on the d_b/H_o' ratio and the slope directly seaward of the breaking point (slope section 2 in Figure A-1).
2. Determine which curves will be utilized according to the H_o'/T^2 ratio.
3. Compute the cotangent of the hypothetical slope which is a straight line from the breaking point to the runup limit computed in the last iteration. The hypothetical slope is shown in Figure A-1.

4. Enter the curve found in Step 2 with the slope found in Step 3 to determine the relative runup values of R/H_0' .
5. Interpolate between curves to find the unique R/H_0' for computed H_0'/T^2 and hypothetical slope.
6. The runup $R = H_0' * R/H_0'$
7. Correct for surface roughness on the slopes above the stillwater line.
8. If the roughness correction factor is 1.00, then the runup must be corrected for scale-effect between model and prototype. The scale effect factor, K , is a function of the hypothetical slope.
9. If the calculated runup minus the runup from the previous iteration is greater than 0.1 ft., then return to Step 3 with the newly calculated runup. Otherwise the iterative process is finished.

A 3.2 Block Diagram

A summary illustration of the program's major steps is presented in a block diagram on the next page.

WAVE RUNUP PROGRAM BLOCK DIAGRAM



A.3.3 Program Description

The wave runup program consists of a main program that controls the activities of eight task orientated subroutines. The components of the program are described in a logical sequence below.

- MAIN - The mainline controls the program activities. It first calls subroutine INPUT to read in the profile information. Upon returning from INPUT, MAIN reads the first set of wave and stillwater level parameters and adjusts the profile elevations so that the zero line is at the stillwater line. Then MAIN calls subroutine RUN to compute the runup. When the runup is calculated, MAIN prints it out along with the wave input information. MAIN then begins again by reading a new set of waves. As many wave and water level conditions as needed can be executed for a given profile. When all the waves for one profile have been executed, the next profile is read and its associated waves are used to compute runup values. The program stops when it runs out of wave parameter cards at the end of the last profile. Several profiles may be executed in one run provided sufficient computation time is allocated in the job control language.
- INPUT - Subroutine INPUT reads the discretized profile information which consists of depth, length, and roughness values. A maximum of fifteen profile points can be read into the program. The profile is read starting at the most seaward point and proceeds landward.

The cotangent of each slope section between profile points is computed. A last landward slope extending inland from the most landward point is also read in as a safety factor so that a computed runup will not overtop the profile causing numerical mistakes. Before returning to the MAIN program, subroutine INPUT prints out the profile points, roughness factors, and slopes.

BLOCK - Subroutine BLOCK DATA contains the data which has been discretized
DATA from Stoa's (1978b) runup charts. The data base is contained in two common blocks; SY and SZ.

Common/SY/ contains three one-dimensional arrays consisting of table entry values. The runup charts require that three variables, cotangent of the slope, H_o'/T^2 and d/H_o' , must be known in order to find a particular R/H_o' value. The three arrays PDB, PDB1, and PCH in SY store the table values for the three variables; slope, H_o'/T^2 and d/H_o' .

Common/SZ/ contains a series of one-dimensional arrays that contain the R/H_o' values for all seven of Stoa's (1978b) runup charts used in the program. The data in this common block is accessed from subroutine RUN by using a three-dimensional array, $DB(PDB, PDB1, PCH)$. Each 1-D array is titled with a D followed by a three-digit number. The first digit represents the family of curves and the second two digits indicate the curve number within the family in ascending order of H_o'/T^2 . For example, D211 is the

eleventh curve in the second family of curves. A " - 999" in the chart indicates no data is available for that particular slope.

RUN - Subroutine RUN calculates the runup of a wave on a given profile based upon the wave characteristics and the profile configuration. The parameters needed for the runup computations are described below.

The breaking depth, DC, is computed based on Weggel's (1972) wave breaking analysis. The distance from the breaking point to the origin (0.0 ft.) is also found. This distance, called DLE, is found by a linear interpolation using the two endpoints of the slope that contain the wave breaking point.

Wave steepness (HOT2) is computed by;

$$\text{HOT2} = \text{HO}/\text{T}^2$$

The value of DC/H_0' is sorted against bounds contained in PCH from which the applicable family of curves is chosen. The distance from the reference point to the runup limit, DSL, is calculated and is used along with variables DLE, DC, and R to compute the hypothetical slope, DCS. Using the computed hypothetical slope, the slope table, PDB, is entered to come out with pointers into the R/H_0' table, DB. Four pointers are found for each case because there are two table values of slope surrounding the hypothetical slope and two H_0'/T^2 table values surrounding the

computed H_o'/T^2 for each slope value. In effect, a parallelogram surrounds the sought after R/H_o' value as shown in Figure A-2. In order to find R/H_o' a log-log interpolation is done between points 1 and 2 to get point 5 and between points 3 and 4 to get point 6. A third interpolation between points 5 and 6 gives the R/H_o' answer.

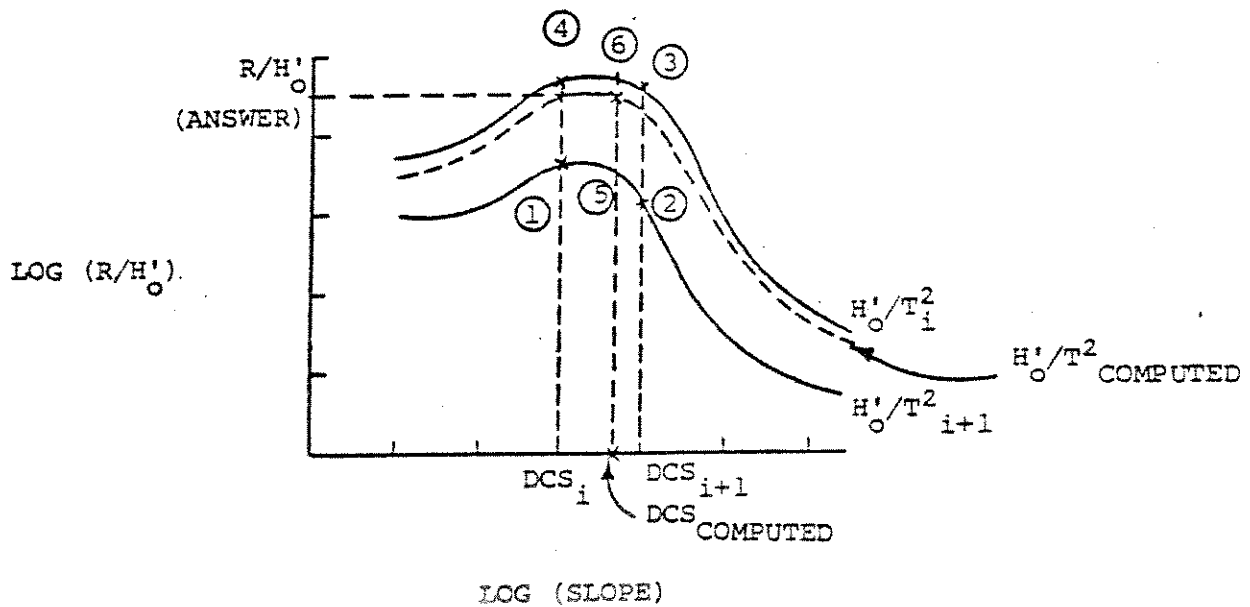


Figure A-2. R/H_o' vs Slope

At times the hypothetical slope is beyond the range of the curve, in which case a log-log extrapolation finds R/H_o' . Referring to Figure A-3, an extrapolation is performed from the last known point and the third point before the last known point. For

example, points 1 and 2 are extrapolated out to point 3 and points 4 and 5 are extrapolated to point 6.

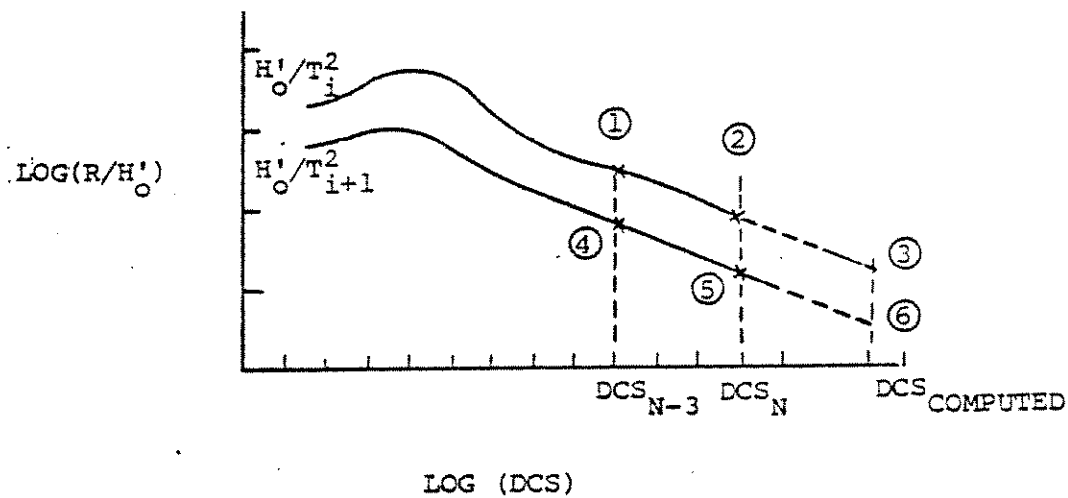


Figure A-3. Extrapolation Example

A final interpolation between points 3 and 6 finds the answer. Once an R/H_o' value is calculated, it is multiplied by H_o' to find the runup, R . Subroutine RRUFF is then called to calculate a roughness correction that is multiplied by R . If the slope is smooth, i.e., the roughness correction factor = 1.0, then the runup, R , is multiplied by a model to prototype scale correction factor. The scale effect is a function of the hypothetical slope and varies from 1.000 to 1.140 (Stoa, 1978b).

If after the first iteration the runup does not overtop the first slope, then the final runup has been found. If the first slope is overtopped, run uses the composite slope method and iterates until

an R is converged on. If after ten iterations the computed runup has not converged, an error flag is tripped and an error message alerting the user to the problem is printed.

LOOK - Subroutine LOOK is a general purpose subroutine designed to locate an input variable between two elements of a table of elements. Input to the subroutine consists of a table of values in an array, the number of elements in the array, and the value to be placed in the table. The output consists of a pointer to the table values above and below the input value. A flag is provided that signals when the input value is out of the range of the table.

RINT - Subroutine RINT executes a linear interpolation given two known values and a third independent value. RINT is used to find the horizontal distance from the reference point to the breaking point and from the reference point to the runup limit.

LOGLOG - Subroutine LOGLOG performs a log-log interpolation given two known values and a third independent value. LOGLOG is used for interpolations on the R/H_0 vs. DCS charts.

LOGLIN - Subroutine LOGLIN executes a log-linear interpolation for cases where the functional relationship has a logarithmic abscissa and a linear ordinate. LOGLIN is used for interpolations on the scale effect factor vs. slope chart.

RRUFF - Each slope section on the profile has a corresponding roughness factor. Subroutine RRUFF computes a weighted average of these roughness factors based on slope section length. The weighted average is evaluated from the stillwater line up to the estimated runup limit only. When the profile data points are input to the program, a roughness factor is also input with the data points.

A.4 Operating Instructions

A.4.1 Program Input

There are several input cards, the first two containing information about the client, engineer, and project followed by a card containing the Last Slope, and cards containing the Profile, Water Levels, and Wave Parameters as follows:

Name Card (A Format)

Col. 1-2 blank

Col. 3-28 contain client's name

Col. 29-60 blank

Col. 61-70 contain engineer's name

Col. 71-80 job number

Job Description Card (A Format)

Col. 1-2 blank

Col. 3-76 describe the project or identify the run

Col. 77-80 contain the run number

Note: The Name Card and the Job Description Card must appear before each profile.

Last Slope Card

Col. 1-4 contain the slope (cotangent) of the profile landward of the most landward point read into the program. (F4.1)

Profile Cards (not more than 15)

Each card contains one point of the beach profile and one slope section roughness factor. The profile points must appear in consecutive order from seaward to landward.

Col. 1 flag IC. The last point on the profile (most landward) is indicated by a 1. (I1)
If not the last point leave blank.

Col. 2 blank

Col. 3-7 depth with respect to a vertical datum (usually NGVD) in feet. (F5.1)

Col. 8 blank

Col. 9-13 horizontal distance from the 0.0 point of the vertical datum to the point of interest in feet, positive for distance landward and negative for distance seaward. (F5.0)

*The reference point is usually taken as the most landward point on the profile but it may be any point on the profile.

Col. 15-19 The roughness factor on the first profile card is the roughness for the slope between the first two sets of profile points. The roughness factor on the second card is the roughness for the slope section between the second and third sets of profile points read into the program. The same logic continues to the last profile card which contains the roughness for the most landward slope section which was read on the Last Slope card. (F5.3)

Water Level and Wave Parameter Cards

There may be as many waves run as desired.

Col. 1 The last parameter card for a given profile is indicated by a 1. This flag notifies the program that another profile is following. Therefore, Col. 1 of the last parameter card of the last profile is left blank (11).

Col. 2-6 stillwater level above vertical datum or NGVD, in feet. (F5.2)

Col. 7 blank

Col. 8-12 Deepwater wave height in feet. (F5.2)

Col. 13 blank

Col. 14-18 wave period in seconds. (F5.2)

In columns 20-24, 26-30, 32-36, 38-42, 44-48, 50-54, 56-60, 62-66, 68-72, and 74-78 a value of breaker height is entered for each slope section (F5.2). Breaker heights are entered starting at the most seaward slope section and proceeding landward. A maximum of ten breaker heights may be input for each deepwater wave. Therefore, only ten slope sections may be below the stillwater level on one profile.

A.4.2 Program Output

Output from the program consists of a printout of the profile, the input wave parameters and runup information. The profile is listed as a set of profile points, cotangents of the slope sections, and the roughness factors for the slopes. The wave parameters printed are the stillwater level, the deepwater wave height and the associated period. The runup output lists the sequence number of the slope section on which the incoming wave breaks, the sequence number of the slope section that the runup limit reaches and the value of runup itself. The sequence number of the slope sections as shown in Figure A-1, starts as 1 at the most seaward slope section and is increased by 1 working landward. The above output is repeated for each profile input to the program.

A.4.3 Error Messages

There are four error messages that alert the user to the most common problems encountered in the program.

SOLUTION DOES NOT CONVERGE - After ten iterations the generated runup and the previously generated runup differ by more than 0.1 ft. The two runups are printed along with the error message. The program is usually oscillating between these two runups when the solution does not converge.

DATA EXCEEDED TABLE - An entry into subroutine LOOK is not within the bounds of the table in which the value is to be placed.

NEGATIVE RUN PARAMETER, PROGRAM STOPS - An input value of deep water wave height or wave period is negative or zero.

MORE THAN 15 POINTS IN PROFILE PROGRAM STOPS - The program can handle a maximum of fifteen discrete profile points only. The program terminates if more than 15 points are input.

A.4.4 Sample IBM Job Control Language

```
1 (Job Card)
1 // MSGLEVEL=(1,1), MSGCLASS=A
1 /*JOBPARM R=300K, T=1, MAIL=YES
```

```
1 //EN057 EXEC PGM=MOD5,REGION=100K, TIME=(,10)
1 //STEPLIB DD DSN=S0952MW.WAVE.LOAD, DISP=(SHR,KEEP)
1 //FT06F001 DD SYOUT=*
1 //SYSUDUMP DD DUMMY
1 //FT05F001 DD *
```

A.5 Sample Problem

A.5.1 Problem Description

The sample case illustrates the input data format and the output results. The profile used in the sample case comes from a transect measured at Old Orchard Beach, Maine (Figure 6-1, Main Text). This is a typical profile configuration for a natural sand beach on an open coastline. The elevations on the profile are referenced to NGVD. The stillwater level represents the level computed for the 100-yr storm surge level. The wave period is the significant period for the 100-yr storm. Several wave heights ranging from 5 to 13 ft. are tried to determine which wave yields the highest runup (see Sec. 6.3, Main Text).

A.5.2 Sample Input

The printout below, generated by Stone & Webster's Input Data Traceback subroutine, illustrates the input data format for the sample case.

INPUT DATA TRACEBACK

DATA SUBMITTED ON DEVICE: 5

CARD#	1	2	3	4	5	6	7	8
1	FEHA					HCH		1296800
2	WAVE RUNUP	TRANSECT 71, EXAMPLE CASE, T=14SEC.						50
3	100							
4	-22.3	-2420	1.0					
5	-16.3	-1129	1.0					
6	-10.3	-587	1.0					
7	-4.3	-170	1.0					
8	00.0	0	1.0					
9	5.0	40	1.0					
10	10.0	120	1.0					

CARD#	1	2	3	4	5	6	7	8
11	15.0	300	1.0					
12	9.1	5.0	14.0	7.2	6.0	6.3	6.9	10.8
13	9.1	7.0	14.0	9.7	10.5	10.6	11.5	13.5
14	9.1	9.0	14.0	11.3	12.5	12.6	13.7	16.4
15	9.1	11.0	14.0	13.4	14.7	15.0	16.0	19.0
16	9.1	13.0	14.0	15.1	16.6	17.0	18.0	21.3
17	6.2	5.0	14.0	7.2	6.0	6.3	6.9	10.8
18	6.2	7.0	14.0	9.7	10.5	10.6	11.5	13.5
19	6.2	9.0	14.0	11.3	12.5	12.6	13.7	16.4
20	6.2	11.0	14.0	13.4	14.7	15.0	16.0	19.0

CARD#	1	2	3	4	5	6	7	8
21	8.2	13.0	14.0	15.1	16.6	17.0	18.0	21.3
								20.0

***** END OF INPUT ON DEVICE 5 *****

A.5.3 Sample Output

The sample problem output as it appears in the computer printout is shown on the next two pages.

CROSS SECTION PROFILE

	LENGTH	ELEV.	SLOPE	ROUGHNESS
1	-2420.0	-22.3	215.17	1.00
2	-1129.0	-16.3	90.33	1.00
3	-567.0	-10.3	69.50	1.00
4	-170.0	-4.3	39.53	1.00
5	0.0	0.0	8.00	1.00
6	40.0	5.0	16.00	1.00
7	120.0	10.0	36.00	1.00
8	300.0	15.0		

LAST SLOPE 10.00 LAST ROUGHNESS 1.00

 OUTPUT TABLE

INPUT PARAMETERS				RUNUP RESULTS		
WATER LEVEL ABOVE DATUM (FT.)	DEEP WATER HAVE HEIGHT (FT.)	HAVE PERIOD (SEC.)	BREAKING SLOPE NUMBER	RUNUP SLOPE NUMBER	RUNUP ABOVE WATER LEVEL (FT.)	
9.1	5.0	14.00	5	7	3.5	
9.1	7.0	14.00	4	7	3.7	
9.1	9.0	14.00	3	7	3.4	
9.1	11.0	14.00	3	7	2.9	
9.1	13.0	14.00	3	7	2.7	
8.2	5.0	14.00	4	7	3.9	
8.2	7.0	14.00	4	7	3.7	
8.2	9.0	14.00	3	7	3.1	
8.2	11.0	14.00	3	7	2.6	
8.2	13.0	14.00	3	7	2.6	

A.6 Program Listing

The FORTRAN source listing of the Wave Runup Program is presented on the following pages.

START COL	1	2	3	4	5	6	7	8
6	233	233	233	233	233	233	233	BLOC1090
6	233	230	225	218	165	135	115	BLOC1100
6	88	78	64	53	36	-999	-999	BLOC1110
6	203	203	203	203	203	205	206	BLOC1120
6	208	205	200	190	146	122	102	BLOC1130
6	76	68	56	46	32	-999	-999	BLOC1140
1	C							
7	DATA D301/							
6	460	460	460	460	460	460	465	BLOC1150
6	465	470	480	490	495	500	505	BLOC1160
6	480	470	430	370	315	235	-999	BLOC1170
6	395	395	400	400	400	395	395	BLOC1180
6	395	400	410	420	420	415	400	BLOC1190
6	365	345	315	270	230	170	-999	BLOC1200
6	330	330	340	340	340	350	355	BLOC1210
6	380	390	400	405	408	390	370	BLOC1220
6	310	290	250	220	185	140	-999	BLOC1230
6	310	315	320	330	330	335	340	BLOC1240
6	370	400	400	400	390	360	365	BLOC1250
6	285	260	220	185	160	120	-999	BLOC1260
6	290	300	305	310	320	325	330	BLOC1270
6	390	400	400	390	380	360	345	BLOC1280
6	265	240	195	160	140	105	-999	BLOC1290
6	270	285	295	300	305	310	320	BLOC1300
6	375	390	395	395	380	355	325	BLOC1310
6	240	215	175	145	125	95	-999	BLOC1320
6	260	265	275	290	295	305	315	BLOC1330
6	375	365	390	385	365	330	295	BLOC1340
7	DATA D307/							
6	205	190	159	130	118	86	-999	BLOC1350
6	240	240	245	245	250	260	275	BLOC1360
6	340	350	350	345	330	295	255	BLOC1370
6	175	160	130	110	95	74	-999	BLOC1380
6	210	210	210	215	220	225	235	BLOC1390
6	280	280	280	275	265	235	200	BLOC1400
6	135	120	100	65	73	57	-999	BLOC1410
6	190	190	190	190	195	200	205	BLOC1420
6	245	245	245	240	230	205	175	BLOC1430
6	115	105	85	70	60	46	-999	BLOC1440
6	175	175	175	180	180	185	190	BLOC1450
6	225	225	220	215	205	180	155	BLOC1460
6	105	92	74	62	53	40	-999	BLOC1470
6	165	165	165	165	160	170	185	BLOC1480
6	195	198	195	195	180	160	135	BLOC1490
6	94	82	66	56	47	36	-999	BLOC1500
6	145	145	145	145	145	148	155	BLOC1510
6	170	175	170	165	155	140	120	BLOC1520
6	84	74	60	49	42	34	-999	BLOC1530
1	C							
7	DATA D401/							
6	195	210	225	240	250	255	265	BLOC1540
6	290	290	300	310	320	330	340	BLOC1550
6	360	360	340	310	275	225	-999	BLOC1560
6	195	210	225	240	250	255	265	BLOC1570
6	290	290	300	310	320	330	340	BLOC1580
6	335	320	280	245	215	170	-999	BLOC1590
6	190	200	210	230	240	250	260	BLOC1600
6	190	200	210	230	240	250	260	BLOC1610
6	190	200	210	230	240	250	260	BLOC1620

FIGURE 10

FIGURE 11

START
COL

6	270,	280,	280,	300,	310,	320,	325,	330,	325,	BLOC1630	
6	300,	275,	235,	200,	175,	140,	-999,	-999,	-999,	BLOC1640	
6	190,	200,	210,	230,	240,	250,	250,	260,	260,	BLOC1650	
6	270,	280,	280,	290,	300,	310,	310,	315,	300,	BLOC1660	
6	245,	245,	200,	175,	150,	120,	-999,	-999,	-999,	BLOC1670	
6	180,	190,	200,	220,	230,	235,	240,	255,	265,	BLOC1680	
6	270,	275,	280,	280,	285,	295,	300,	295,	265,	BLOC1690	
6	230,	205,	170,	145,	125,	100,	-999,	-999,	-999,	BLOC1700	
6	170,	180,	185,	190,	205,	220,	230,	235,	250,	BLOC1710	
6	255,	258,	240,	242,	270,	280,	285,	270,	240,	BLOC1720	
6	215,	190,	155,	135,	115,	90,	-999,	-999,	-999,	BLOC1730	
6	155,	165,	175,	180,	185,	195,	200,	210,	220,	BLOC1740	
6	225,	235,	240,	245,	250,	260,	265,	240,	210,	BLOC1750	
7	DATA D407/	165,	140,	120,	103,	80,	-999,	-999,	-999,	BLOC1760	
6	145,	150,	155,	165,	170,	180,	185,	192,	200,	BLOC1770	
6	205,	210,	215,	220,	230,	235,	225,	200,	175,	BLOC1780	
6	155,	140,	115,	98,	66,	66,	-999,	-999,	-999,	BLOC1790	
6	120,	127,	132,	140,	148,	152,	160,	167,	177,	BLOC1800	
6	180,	190,	200,	205,	210,	200,	175,	155,	135,	BLOC1810	
6	125,	110,	90,	77,	67,	53,	-999,	-999,	-999,	BLOC1820	
6	115,	120,	125,	135,	140,	145,	150,	160,	172,	BLOC1830	
6	175,	177,	190,	195,	195,	180,	155,	135,	120,	BLOC1840	
6	106,	97,	80,	67,	50,	44,	-999,	-999,	-999,	BLOC1850	
6	110,	115,	120,	130,	135,	140,	145,	155,	170,	BLOC1860	
6	172,	180,	185,	167,	185,	168,	140,	120,	106,	BLOC1870	
6	93,	84,	69,	58,	50,	39,	-999,	-999,	-999,	BLOC1880	
6	105,	110,	115,	120,	128,	132,	130,	147,	160,	BLOC1890	
6	165,	172,	180,	180,	177,	155,	130,	110,	96,	BLOC1900	
6	85,	75,	62,	52,	45,	35,	-999,	-999,	-999,	BLOC1910	
6	92,	98,	102,	108,	115,	122,	128,	136,	150,	BLOC1920	
6	155,	165,	168,	165,	158,	135,	115,	97,	84,	BLOC1930	
6	75,	65,	53,	44,	37,	29,	-999,	-999,	-999,	BLOC1940	
1	C	FIGURE 2								BLOC1950	
7	DATA D501/	240,	270,	280,	290,	290,	295,	305,	315,	325,	BLOC1960
6	335,	345,	360,	375,	400,	420,	430,	430,	430,	430,	BLOC1970
6	425,	415,	375,	325,	280,	215,	130,	90,	55,	55,	BLOC1980
6	210,	230,	240,	255,	265,	270,	275,	280,	290,	290,	BLOC1990
6	300,	310,	320,	340,	355,	380,	400,	402,	395,	395,	BLOC2000
6	365,	365,	315,	265,	220,	170,	100,	72,	45,	45,	BLOC2010
6	185,	200,	210,	225,	235,	240,	243,	247,	253,	253,	BLOC2020
6	265,	280,	295,	310,	325,	350,	360,	360,	350,	350,	BLOC2030
6	340,	315,	270,	225,	185,	140,	84,	60,	37,	37,	BLOC2040
6	160,	175,	165,	200,	209,	212,	218,	222,	230,	230,	BLOC2050
6	240,	250,	270,	280,	300,	325,	335,	330,	315,	315,	BLOC2060
6	300,	275,	230,	195,	160,	120,	73,	53,	33,	33,	BLOC2070
6	140,	152,	162,	175,	185,	192,	200,	208,	215,	215,	BLOC2080
6	230,	240,	255,	265,	280,	295,	305,	300,	285,	285,	BLOC2090
6	270,	245,	200,	165,	135,	100,	62,	43,	27,	27,	BLOC2100
6	130,	145,	155,	168,	175,	182,	188,	190,	205,	205,	BLOC2110
6	215,	230,	240,	250,	260,	260,	268,	260,	265,	265,	BLOC2120
6	245,	220,	180,	145,	120,	92,	56,	40,	26,	26,	BLOC2130
6	130,	140,	148,	158,	165,	170,	175,	182,	190,	190,	BLOC2140
6	205,	215,	230,	240,	250,	265,	270,	260,	240,	240,	BLOC2150
7	DATA D507/	215,	190,	160,	130,	110,	82,	50,	37,	24,	BLOC2160

START COL

-----1-----2-----3-----4-----5-----6-----7-----8

6	1	125,	132,	140,	150,	155,	160,	165,	170,	180,	BLOC2170
6	2	190,	203,	215,	225,	235,	242,	240,	225,	200,	BLOC2180
6	3	180,	155,	125,	105,	90,	68,	44,	36,	23,	BLOC2190
6	1	120,	127,	132,	140,	145,	150,	152,	160,	168,	BLOC2200
6	2	178,	190,	200,	207,	213,	207,	190,	170,	150,	BLOC2210
6	3	135,	120,	100,	85,	72,	56,	37,	28,	19,	BLOC2220
6	1	115,	122,	125,	135,	140,	145,	147,	152,	160,	BLOC2230
6	2	170,	180,	192,	200,	203,	190,	165,	145,	125,	BLOC2240
6	3	112,	100,	84,	72,	62,	48,	32,	24,	17,	BLOC2250
6	1	115,	122,	125,	135,	140,	145,	147,	152,	160,	BLOC2260
6	2	170,	180,	190,	195,	195,	180,	150,	130,	115,	BLOC2270
6	3	100,	90,	72,	62,	53,	40,	28,	22,	16,	BLOC2280
6	1	115,	122,	125,	135,	140,	145,	147,	152,	160,	BLOC2290
6	2	170,	180,	190,	190,	190,	165,	142,	118,	102,	BLOC2300
6	3	90,	80,	65,	55,	47,	36,	25,	19,	15,	BLOC2310
6	1	110,	115,	120,	128,	132,	138,	140,	150,	155,	BLOC2320
6	2	165,	175,	185,	185,	180,	155,	125,	105,	90,	BLOC2330
6	3	78,	68,	55,	47,	40,	32,	22,	17,	13,	BLOC2340
1	C	FIGURE 3									
1	7	DATA D601/									
6	2	250,	265,	275,	290,	300,	310,	315,	335,	365,	BLOC2350
6	3	380,	405,	420,	425,	430,	438,	439,	435,	432,	BLOC2360
6	3	432,	430,	420,	380,	320,	245,	145,	100,	60,	BLOC2370
6	1	180,	190,	195,	205,	210,	215,	220,	230,	240,	BLOC2380
6	2	250,	265,	280,	295,	315,	340,	350,	360,	360,	BLOC2390
6	3	360,	350,	320,	270,	220,	170,	100,	70,	42,	BLOC2400
6	1	150,	160,	162,	170,	178,	177,	180,	168,	195,	BLOC2420
6	2	210,	240,	260,	270,	280,	295,	305,	305,	305,	BLOC2430
6	3	290,	275,	245,	210,	175,	135,	82,	57,	35,	BLOC2440
6	1	135,	140,	142,	145,	150,	152,	155,	162,	175,	BLOC2450
6	2	185,	215,	240,	250,	255,	270,	275,	280,	275,	BLOC2460
6	3	260,	250,	205,	175,	150,	110,	70,	50,	32,	BLOC2470
6	1	128,	130,	132,	135,	140,	142,	145,	150,	165,	BLOC2480
6	2	175,	200,	225,	230,	240,	250,	255,	260,	260,	BLOC2490
6	3	230,	215,	175,	145,	125,	100,	62,	44,	28,	BLOC2500
6	1	125,	128,	130,	133,	138,	140,	142,	149,	155,	BLOC2510
6	2	162,	190,	210,	215,	220,	230,	240,	242,	235,	BLOC2520
6	3	210,	190,	150,	135,	115,	90,	56,	41,	26,	BLOC2530
6	1	125,	128,	130,	135,	138,	140,	142,	150,	155,	BLOC2540
6	2	162,	190,	200,	202,	210,	225,	230,	225,	210,	BLOC2550
6	3	190,	170,	140,	120,	100,	72,	53,	36,	25,	BLOC2560
6	1	125,	128,	130,	135,	138,	140,	142,	150,	155,	BLOC2570
6	2	165,	180,	190,	202,	210,	215,	215,	200,	180,	BLOC2580
6	3	160,	140,	120,	105,	92,	72,	48,	35,	23,	BLOC2590
6	1	118,	120,	121,	122,	125,	128,	132,	135,	142,	BLOC2600
6	2	155,	170,	185,	195,	200,	202,	185,	160,	140,	BLOC2610
6	3	120,	110,	92,	77,	68,	55,	37,	28,	19,	BLOC2620
6	1	115,	116,	117,	119,	120,	121,	125,	135,	147,	BLOC2630
6	2	155,	165,	175,	185,	190,	190,	175,	145,	120,	BLOC2640
6	3	110,	97,	80,	70,	60,	47,	32,	29,	17,	BLOC2650
6	1	115,	116,	117,	119,	120,	121,	125,	135,	147,	BLOC2660
6	2	155,	165,	175,	185,	190,	180,	150,	130,	110,	BLOC2670
6	3	95,	85,	70,	60,	51,	41,	28,	22,	16,	BLOC2680
6	1	115,	116,	117,	119,	120,	121,	125,	135,	147,	BLOC2690
6	2	152,	162,	175,	180,	180,	165,	140,	117,	100,	BLOC2700

DATA D607/

DATE: 01/10/26
TIME: 11:13
PAGE: 06 OF 06

MEMBER: BLOCK
LEVEL: 01.03
USERID: S4692NN

PROJECT: 20052701
LIBRARY: EN57
TYPE: DATA3
START COL

COL	1	2	3	4	5	6	7	8
6	85	75	63	52	45	35	25	20, 15, BLOC2710
6	105	106	109	111	115	118	122	132, 143, BLOC2720
6	150	162	168	169	164	145	125	108, 90, BLOC2730
6	78	68	56	48	40	32	23	18, 14, BLOC2740
1	C							
7	FIGURE 4							
6	200	205	210	210	215	220	230	250, 270, BLOC2750
6	280	290	315	325	345	365	380	395, 405, BLOC2760
6	408	412	408	360	310	230	145	100, 62, BLOC2780
6	145	147	148	149	150	152	154	165, 162, BLOC2790
6	200	230	255	262	270	280	290	295, 300, BLOC2800
6	305	310	300	259	215	165	100	70, 43, BLOC2810
6	130	130	130	130	133	135	137	145, 160, BLOC2820
6	175	205	220	230	245	255	265	270, BLOC2830
6	270	265	240	205	180	135	85	59, 36, BLOC2840
6	120	120	120	122	125	128	130	137, 145, BLOC2850
6	165	190	210	220	226	235	245	250, 248, BLOC2860
6	240	225	195	165	145	115	74	52, 33, BLOC2870
6	120	120	120	122	125	128	130	137, 145, BLOC2880
6	160	160	190	200	210	225	235	225, BLOC2890
6	215	200	175	150	130	105	66	47, 30, BLOC2900
6	120	120	120	122	125	128	130	137, 145, BLOC2910
6	160	170	185	195	200	210	220	225, 210, BLOC2920
6	195	180	155	135	120	95	60	44, 27, BLOC2930
6	120	120	120	122	125	128	130	137, 145, BLOC2940
6	155	165	160	190	200	215	220	210, 195, BLOC2950
7	DATA D701/							
6	175	160	135	115	100	80	53	39, 26, BLOC2960
6	110	112	114	118	120	122	125	132, 142, BLOC2970
6	150	160	170	180	190	200	205	180, 160, BLOC2980
6	145	135	112	98	83	67	44	33, 23, BLOC2990
6	105	107	109	112	115	119	122	126, 135, BLOC3000
6	145	155	165	175	180	190	180	155, 135, BLOC3010
6	120	108	90	75	66	52	37	29, 20, BLOC3020
6	105	107	109	112	115	119	122	126, 132, BLOC3030
6	140	152	162	170	180	180	155	135, 115, BLOC3040
6	105	92	76	66	56	46	35	25, 18, BLOC3050
6	105	107	109	112	115	119	122	126, 132, BLOC3060
6	138	150	160	168	175	168	140	120, 105, BLOC3070
6	92	82	70	58	52	40	29	23, 17, BLOC3080
6	105	107	109	112	115	119	122	126, 132, BLOC3090
6	140	150	160	170	170	155	130	110, 95, BLOC3100
6	65	76	63	53	43	37	27	22, 16, BLOC3110
6	100	102	106	110	112	116	118	122, 127, BLOC3120
6	133	148	156	165	165	140	115	98, 83, BLOC3130
6	75	65	54	45	40	31	23	19, 14, BLOC3140
7	END							


```

TART
COL 1-----2-----3-----4-----5-----6-----7-----8-----
7 SUBROUTINE INPUT
1 C THIS ROUTINE INPUTS HEADING DATA, LAST SLOPE, AND PROFILE
1 C AND PRINTS INPUT
1 C
1 C*****VARIABLE DICTIONARY
1 C
1 C NAME MODE SIZE DESCRIPTION UNITS
1 C
1 C DEP I*2 16 VERT DIMENSION OF PROFILE FT*10
1 C DL I*2 16 HORIZONTAL DIMENSION OF PROFILE FT
1 C S R*4 16 PROFILE SLOPE VALUES
1 C NP I*4 1 NUMBER OF POINTS IN PROFILE
1 C HTL I*2 1 WATER LEVEL FT*10
1 C IPAGE I*4 1 CURRENT PAGE NUMBER
1 C DT I*2 110 PAGE HEADING
1 C RDEP R*4 16 DEPTH INPUT BUFFER, LAND TO SEA FT
1 C RFLAT R*4 16 LENGTH INPUT BUFFER, LAND TO SEA FT
1 C FLAT A*4 1 ALPHANUMERIC CONSTANT 'FLAT'
1 C BLANK A*4 1 ALPHANUMERIC CONSTANT ' '
1 C SL R*4 1 SLOPE OF LAST LANDWARD SECTION
1 C IC I*4 1 FLAG TO DETECT END OF PROFILE DATA
1 C GA R*4 1 USED IN SLOPE CALCULATIONS
1 C ROD R*4 1 OUTPUT BUFFER OF LENGTHS FT
1 C RDP R*4 1 OUTPUT BUFFER OF DEPTHS FT
1 C SI R*4 1 OUTPUT BUFFER OF SLOPES
1 C ROUGH R*4 16 ARRAY OF ROUGHNESS VALUES
1 C
1 C*****START OF SUBROUTINE
7 IMPLICIT INTEGER*2(D,P)
7 INTEGER*2 HTL
7 COMMON /HD/ DEPI(16),DL(16),S(16),HB(16),ROUGH(16),NP,NTL
7 DIMENSION DT(110),RDEP(16),RDL(16)
7 DATA FLAT,BLANK/'FLAT',' /
7 EQUIVALENCE (RDEP(1),S(1))
1 C-----READ PAGE HEADING DATA
7 READ(5,1000) (DT(I),I=5,17),(DT(11),I=47,51),(DT(I),I=55,59),
6 1 (DT(I),I=64,100),DT(112),DT(113)
1 C-----WRITE HEADING DATA
7 IPAGE = IPAGE+1
7 WRITE(6,1100)DT,IPAGE
1 C-----READ SLOPE, DEFAULT=0 IF SLOPE .LT. 0
4 10 READ (5,1200) SL
7 IF (SL.LT.0) SL=0.
1 C-----READ IN PROFILE ALL DIMENSIONS ARE IN FEET
7 DO 20 NP=1,15
7 READ (5,1300) IC,RDEP(NP),RDL(NP),ROUGH(NP)
7 IF (IC.EQ.1) GOTO 30
4 20 CONTINUE
1 C-----TOO MANY SLOPES IN INPUT
7 WRITE (6,1400)
7 STOP
1 C-----FILL UP DEP,DL,ROUGH ARRAYS
4 30 II=NP
  
```

```

1  C-----1-----2-----3-----4-----5-----6-----7-----8
2L -----1-----2-----3-----4-----5-----6-----7-----8

```

```

7 NP=NP+1
7 DO 40 J=1,II
7 DEP(J)=RDEP(J)*10.+ SIGN(0.1,RDEP(J))
4 DL(J)=RDL(J)
7 S(II)=SL
1 C-----CALCULATE SLOPES
7 NA=NP-2
7 DO 50 I=1,NA
7 GA=(DEP(I+1)-DEP(I))/10.
7 IF(CASS(GA).LT.0.0001)GA=0.0001
4 S(I)=(DL(I+1)-DL(I))/GA
7 DEP(NP)=DEP(II)+1000
7 DL(NP)=S(II)*(DEP(NP)-DEP(II))/10
1 C-----PRINT OUT PROFILE
7 WRITE (6,1500)
7 DO 60 I=1,II
7 RDS=DL(I)
7 RDP=DEP(I)/10.
7 S1=S(I)
7 KRI=ROUGH(I)
7 IF(S1.GT.1000) S1=FLAT
7 IF(I.EQ.II) S1=BLANK
7 IF(I.EQ.II) RRI=BLANK
7 WRITE (6,1900) I,RDD,RDP
7 IF(S1.NE.S(II)) GOTO 60
7 WRITE (6,1600) S1,RRI
7 GOTO 80
4 60 IF(RRI.NE.ROUGH(II)) GO TO 70
7 WRITE (6,1700) S1,RRI
7 GO TO 60
4 70 WRITE(6,1800) S1,RRI
4 80 CONTINUE
7 WRITE (6,2000) S(II),ROUGH(II)
7 RETURN
2 1000 FORMAT(2X,I3A2,32X,10A2/2X,39A2)
2 1100 FORMAT('1',59A2/'0',59A2,I19,I2//,60('**')//)
2 1200 FORMAT (F4.1)
2 1300 FCRIAT(I1,IX,F5.1,IX,F5.0,IX,F5.3)
2 1400 FORMAT(' MORE THAN 15 POINTS IN PROFILE, PROGRAM STOPS.')
2 1500 FCRIAT(I23,'CROSS SECTION PROFILE.
6 1 //I21,'LENGTH ELEV. SLOPE ROUGHNESS'/)
2 1600 FCRIAT(I38,F7.2,I51,F5.2)
2 1700 FCRIAT(I41,A4,I51,F5.2)
2 1800 FCRIAT(I41,A4,I51,A4)
2 1900 FCRIAT(I1X,I10,I2,I20,F7.1,I30,F5.1)
2 2000 FCRIAT('0',I26,'LAST SLOPE',F7.2,'
7 LAST ROUGHNESS',F7.2)
7 END

```

```

INPU0560
INPU0570
INPU0590
INPU0600
INPU0610
INPU0620
INPU0630
INPU0640
INPU0650
INPU0660
INPU0670
INPU0680
INPU0690
INPU0700
INPU0710
INPU0720
INPU0730
INPU0740
INPU0750
INPU0760
INPU0770
INPU0780
INPU0790
INPU0800
INPU0810
INPU0820
INPU0830
INPU0840
INPU0850
INPU0860
INPU0870
INPU0880
INPU0890
INPU0900
INPU0910
INPU0920
INPU0930
INPU0940
INPU0950
INPU0960
INPU0970
INPU0980
INPU0990
INPU1000
INPU1010
INPU1020
INPU1030

```

MEMBER: LOGLIN
LEVEL: 01.00
USERID: S4692HM

DATE: 01/10/26
TIME: 11:13
PAGE: 01 OF 01

LOGLIN

START
COL

1-----2-----3-----4-----5-----6-----7-----8

```
7 SUBROUTINE LOGLIN(X1,X2,Y1,Y2,X,Y)
1 C
1 THIS SUBROUTINE PERFORMS A LOG-LINEAR INTERPOLATION BETWEEN TWO
1 KNOWN POINTS (X1,Y1) AND (X2,Y2). THE VALUE OF X IS CONTAINED
1 BETWEEN X1 AND X2 ON THE LOGARITHMIC SCALE. THE OUTPUT VALUE,
1 Y IS CONTAINED BETWEEN Y1 AND Y2 ON THE LINEAR SCALE. REAL
1 NUMBERS ENTER THE SUBROUTINE AND THE NECESSARY LOGARITHMS ARE
1 DONE IN THE SUBROUTINE.
7 IMPLICIT INTEGER*2(X,Y)
7 RX1=X1
7 RX2=X2
7 RX=X
7 RX1=ALOG10(RX1)
7 RX2=ALOG10(RX2)
7 RX=ALOG10(RX)
7 SLOPE=(Y1-Y2)/(RX1-RX2)
7 Y=Y1+SLOPE*(RX-RX1)
7 RETURN
7 END
```

LOLI0010
LOLI0020
LOLI0030
LOLI0040
LOLI0050
LOLI0060
LOLI0070
LOLI0080
LOLI0090
LOLI0100
LOLI0110
LOLI0120
LOLI0130
LOLI0140
LOLI0150
LOLI0160
LOLI0170
LOLI0180

-----1-----2-----3-----4-----5-----6-----7-----8

```

7 SUBROUTINE LOGLOG(X1,X2,Y1,Y2,X,Y)
1 C
1 C THIS SUBROUTINE PERFORMS A LOGLOG INTERPOLATION FOR COMPUTED
1 C VALUE X CONTAINED BETWEEN KNOWN VALUES X1 AND X2. THE OUTPUT IS
1 C THE REAL VALUE Y, WHICH IS CONTAINED BETWEEN KNOWN VALUES Y1 AND
1 C Y2. INPUT VARIABLES TO THE SUBROUTINE ARE REAL. THE LOGARITHM
1 C OF EACH VARIABLE IS TAKEN IN THE SUBROUTINE.
1 C
7 IMPLICIT INTEGER*2(X,Y)
7 RX1=X1
7 RX2=X2
7 RY1=Y1
7 RY2=Y2
7 RX=X
7 RX1=ALOG10(RX1)
7 RX2=ALOG10(RX2)
7 RY1=ALOG10(RY1)
7 RY2=ALOG10(RY2)
7 RX=ALOG10(RX)
7 SLOPE=(RY1-RY2)/(RX1-RX2)
7 Y=10**((RY1+SLOPE*(RX-RX1))
7 RETURN
7 END

```

LOLO0010
LOLO0020
LOLO0030
LOLO0040
LOLO0050
LOLO0060
LOLO0070
LOLO0080
LOLO0090
LOLO0100
LOLO0110
LOLO0120
LOLO0130
LOLO0140
LOLO0150
LOLO0160
LOLO0170
LOLO0180
LOLO0190
LOLO0200
LOLO0210
LOLO0220
LOLO0230

DATE: 81/10/26
TIME: 11:13
PAGE: 01 OF 01

MEMBER: LOOK
LEVEL: 01.00
USERID: S4692HW

PROJECT: S0952HM
LIBRARY: EN57
TYPE: DATA3

START COL 1-----2-----3-----4-----5-----6-----7-----8

```

7 SUBROUTINE LOOK(X,N,IV,L,H,IFG) LOOK0010
1 LOOK -- DIGITIZE ANALOG INPUT VALUE BY MODIFIED BINARY SEARCH LOOK0020
1 C OUTPUT POINTERS TO VALUES IMMEDIATELY BEFORE AND AFTER INPUT VALUE LOOK0030
1 C C*****VARIABLE DICTIONARY LOOK0040
1 C LOOK0050
1 C LOOK0060
1 C LOOK0070
1 C LOOK0080
1 C LOOK0090
1 C LOOK0100
1 C LOOK0110
1 C LOOK0120
1 C LOOK0130
1 C LOOK0140
1 C LOOK0150
1 C LOOK0160
1 C LOOK0170
1 C LOOK0180
1 C LOOK0190
1 C LOOK0200
1 C LOOK0210
1 C LOOK0220
1 C LOOK0230
1 C LOOK0240
1 C LOOK0250
1 C LOOK0260
1 C LOOK0270
1 C LOOK0280
1 C LOOK0290
1 C LOOK0300
1 C LOOK0310
1 C LOOK0320
1 C LOOK0330
1 C LOOK0340
1 C LOOK0350
1 C LOOK0360
1 C LOOK0370
1 C LOOK0380
1 C LOOK0390
1 C LOOK0400
1 C LOOK0410
1 C LOOK0420
1 C LOOK0430

```

HAHE HODE SIZE DESCRIPTION

I*2 I*4 1 TABLE TO BE LOOKED INTO (ASCENDING ORDER)

I*2 1 NUMBER OF ELEMENTS IN TABLE

I*4 1 ANALOG INPUT VALUE

I*4 1 POINTER TO ENTRY IN X BEFORE IV

I*4 1 POINTER TO ENTRY IN X AFTER IV

I*4 1 TABLE EXCEEDED FLAG

C*****START OF SUBROUTINE

INTEGER*2 IV,X(1)

L=1

H=N

C-----CHECK TO SEE IF DATA EXCEEDS TABLE

IF(X(L).GT.IV) GOTO 30

IF(X(H)-IV)40,20,20

C-----PERFORM LOOKUP

10 IF(X(H).GT.IV) GOTO 20

C-----MOVE LOW POINTER UP

L=H

H=H0

C-----MOVE HI POINTER DOWN

20 H0=H

H=(H-L)/2+L

C-----CHECK TO SEE IF DONE

IF(H.NE.L) GOTO 10

IF(H.NE.H) H=L+1

RETURN

C-----DATA LESS THAN 1ST ENTRY IN TABLE

30 H=1

IFG=1

RETURN

C----- DATA GREATER THAN LAST ENTRY IN TABLE

40 L=H

IFG=1

RETURN

END

START
COL -----1-----2-----3-----4-----5-----6-----7-----8

```

1 C PROGRAM RUNUP
1 C THIS PROGRAM CALCULATES THE RUNUP OF A HAVRE ON A COMPOSITE SLOPE
1 C
1 C *****VARIABLE DICTIONARY
1 C
1 C NAME MODE SIZE DESCRIPTION UNITS
1 C
1 C DEP I*2 16 VERT DIMENSION OF PROFILE,SEA TO LAND FT*10
1 C DL I*2 16 HORIZ DIMENSION OF PROFILE,SEA TO LAND FT
1 C S R*4 16 SLOPES OF PROFILE
1 C RP I*4 1 NUMBER OF POINTS IN PROFILE
1 C IPAGE I*4 1 CURRENT PAGE NUMBER
1 C DT I*2 118 PAGE HEADING
1 C H R*4 1 HEIGHT OF DEEP-WATER HAVRE FEET
1 C T R*4 1 PERIOD OF DEEP-WATER WAVE SEC
1 C R R*4 1 CALCULATED RUNUP FEET
1 C YI I*4 1 NO. OF SLOPE ON WHICH WAVE BREAKS
1 C ISL I*4 1 NO. OF SLOPE ON WHICH RUNUP LIMIT FALLS
1 C IFC I*4 16 CONVERGENCE FLAGS
1 C IFG I*4 16 EXCEED TABLE FLAG
1 C IFO I*4 1 DUMMY
1 C LISL I*4 16 TABLE OF STARTING SLOPES
1 C LIIL I*4 16 TABLE OF ENDING SLOPES
1 C RAS R*4 16 TABLE OF CALCULATED RUNUPS FEET
1 C HTB R*4 1 INPUT WATER LEVEL FT*10
1 C HTL I*2 1 WATER LEVEL MULTIPLIED BY 10 FT*10
1 C IZ I*4 1 VALUE OF HTL AT PREVIOUS STEP
1 C IH I*4 1 POINTER INTO ARRAYS OF ANSWERS AND FLAGS
1 C DA I*2 25 POINTER TO MAXIMUM RUNUP
1 C
1 C *****START OF PROGRAM
7 IPLICIT INTEGER*2(D,P)
7 INTEGER*2 HTL,HTL
7 COMMON /OUT/ LISL(16),LIIL(16),RAS(16),IFG(16),IFC(16)
7 COMMON /TD/ DEP(16),DL(16),S(16),HB(16),ROUGH(16),NP,HTL
7 COMMON /HD/ IPAGE,DT
7 DIMENSION DA(25),DT(118)
7 DATA DA/ ' ','SO','LU','TI','ON','D','OE','S','HO','Y','CO',
6 'LV','ER','GE',' ','DA','TA','E','XC','EE','DE','D','TA','BL',
6 'E'/'
1 C READ IN PROFILE
1 10 CALL INPUT
7 IJK=0
7 HTL=0
7 K=0
7 IPAGE =IPAGE+1
7 WRITE(6,1100) DT,IPAGE
1 C OUTPUT TABLE
7 WRITE(6,1300)
4 20 IF (K.EQ.1)GO TO 10
7 IF (IJK.GT.0) HTL=HTB*10
7 K=K+5,1000,END=700K,HTB,H0,T,(HB(I),I=1,10)
1 C BRANCH ON NEGATIVE RUN PARAMETERS

```

-----1-----2-----3-----4-----5-----6-----7-----8

START COL 1 2 3 4 5 6 7 8

```

7 IF(H0.LE.0.OR.T.LE.0) GOTO 60
7 WTL=WTB*10.
7 IQ=1
1 C REFERENCE PROFILE TO STILL WATER LEVEL
7 DO 30 I=1,NP
4 30 DEP(I)=DEP(I)-WTL+HTL
7 IJK=IJK+1
7 WTL=0
4 40 CALL RUK(H0,T,R,II,IQ)
1 C
7 IH=IQ-1
7 IF(IFG(IQ).EQ.1) GO TO 60
7 IF(IFC(IQ).EQ.1) GO TO 50
7 WRITE (6,1400) WTB,H0,T,LII(IQ),LISL(IQ),RAS(IQ)
7 GO TO 20
4 50 WRITE (6,1500) WTB,H0,T,LII(IQ),LISL(IQ),RAS(IH),RAS(IQ),(DA(J),J=HAIH0700
6 1 1,14)
7 GO TO 20
4 60 WRITE (6,1600) WTB,H0,T,(DA(J),J=1,25)
7 GOTO 20
4 70 STCP
4 80 WRITE (6,1200)
7 STOP
2 FCRIAT(11,F5.2,12(1X,F5.2))
2 1100 FCRIAT('1',59A2/'0',59A2,11A,12//,60('**')//)
2 1200 FORHAT(' NEGATIVE RUN PARAMETER, PROGRAM STOPS.')
2 1300 FORHAT('45','OUTPUT TABLE',T65,6('---')//T20,'INPUT PARAMETERS',
6 1169,'RUNUP RESULTS',T20,8('---'),T69,13('---')//T9,'WATER LEVEL',
6 2124,'DEEP WATER',T58,'BREAKING SLOPE',T76,'RURUP SLOPE',T91,
6 3'RURUP ABOVE',T19,'ABOVE DATUM',T24,'HAVE HEIGHT',T39,
6 4'HAVE PERIOD',T62,'NUMBER',T79,'NUMBER',T91,'WATER LEVEL',T12,
6 5'(FT.)',T27,'(FT.)',T42,'(SEC.)',T94,'(FT.)')
2 1400 FORHAT('110,F6.1,T25,F6.1,T40,F6.2,T69,T2,T91,I2,T95,F4.1)
2 1500 FORHAT('110,F6.1,T25,F6.1,T40,F6.2,T64,I2,T61,I2,T69,F4.1,
6 1195,F4.1,T99,I4A2)
2 1600 FORHAT('110,F6.1,T25,F6.1,T40,F6.2,T55,25A2)
7 END

```

HAIH0550
HAIH0560
HAIH0570
HAIH0580
HAIH0590
HAIH0600
HAIH0610
HAIH0620
HAIH0630
HAIH0640
HAIH0650
HAIH0660
HAIH0670
HAIH0680
HAIH0690
HAIH0700
HAIH0710
HAIH0720
HAIH0730
HAIH0740
HAIH0750
HAIH0760
HAIH0770
HAIH0780
HAIH0790
HAIH0800
HAIH0810
HAIH0820
HAIH0830
HAIH0840
HAIH0850
HAIH0860
HAIH0870
HAIH0880
HAIH0890
HAIH0900
HAIH0910

```

START -----1-----2-----3-----4-----5-----6-----7-----8
COL  +-----+-----+-----+-----+-----+-----+-----+-----+
1  SUBROUTINE RINT(X1,X2,Y1,Y2,X,Y)
7  RINT -- SINGLE LINEAR INTERPOLATION BY METHOD Y=HX+B
1  C  INPUT KNOWN DATA POINTS (X1,Y1),(X2,Y2)
1  C  GIVEN X FIND Y=F(X)=HX+B  H=SLOPE  B=START VALUE
1  C  OUTPUT (X,Y)
1  C
1  C****VARIABLE DICTIONARY
1  C
1  C  ALL INPUT AND OUTPUT IS I*2
1  C
1  C****START OF SUBROUTINE
7  IMPLICIT INTEGER*(X,Y)
7  G=X2-X1
1  C-----DIVISION BY ZERO CHECK
7  IF(G.NE.0.) GOTO 10
7  Y=Y1
7  RETURN
4  10 RAT=(X-X1)/G
7  Y=(Y2-Y1)*RAT+Y1
7  RETURN
7  END
  
```

RINT0010
 RINT0020
 RINT0030
 RINT0040
 RINT0050
 RINT0060
 RINT0070
 RINT0080
 RINT0090
 RINT0100
 RINT0110
 RINT0120
 RINT0130
 RINT0140
 RINT0150
 RINT0160
 RINT0170
 RINT0180
 RINT0190
 RINT0200
 RINT0210


```

ART 1-----1-----2-----3-----4-----5-----6-----7-----8
CL  SUBROUTINE RRUFF(RL,FROUGH,N)
1 C
1 C COMPUTATION OF WEIGHTED ROUGHNESS FACTOR FROM
1 C HTL TO HAVE RURUP LIMIT
1 C
1 C ***** VARIABLE DICTIONARY *****
1 C
1 C NAME    MODE SIZE
1 C
1 C SLPEN R*4 1    DISTANCE ALONG ONE SLOPE
1 C ROUGH R*4 16    ROUGHNESS FACTOR ON ONE SLOPE
1 C FROUGH R*4 1    FINAL ROUGHNESS FACTOR FOR THE TOTAL SLOPE
1 C                    LENGTH FROM HTL TO RAS(III)
1 C R#1 I*4 1    NO. OF POINTS IN THE PROFILE
1 C TOTL#H R*4 1    TOTAL SLOPE LENGTH FROM HTL TO RAS(III)
1 C RL R*4 1    ROUGHNESS FACTOR TIMES SLOPE LENGTH
1 C
7    IMPLICIT INTEGER*2(D,P)
7    INTEGER*2 HTL
7    COMMON /TD/ DEP(16),DL(16),S(16),HB(16),ROUGH(16),NP,HTL
7    N#1=NP-1
7    TOTLEN=0.0
7    TOTRL=0.0
1 C    FIND SLOPE THAT STILL WATER LEVEL INTERSECTS, LI
7    IF(N.GT.1) GO TO 30
7    DO 10 J1=1,N#1
7    IF(HTL.LT.DEP(J1+1)) GO TO 20
4    10 CONTINUE
4    20 LI=J1
4    30 DOIR=HTL+R1*10
1 C    FIND SLOPE THAT RUPUP INTERSECTS, LIS
7    DO 40 J2=1,N#1
7    IF(DDTR.LT.DEP(J2+1)) GO TO 50
4    40 CONTINUE
4    50 LIS=J2
7    DO 60 K=LI,LIS
1 C    FIND LENGTH OF INDIVIDUAL SLOPE SECTION
7    SLPEN=((DEP(K+1)-DEP(K))/10.)*2+DL(K+1)-DL(K)**2)**0.5
1 C    MULTIPLY SLOPE SECTION LENGTH BY ROUGHNESS FACTOR
7    RL=SLPEN*ROUGH(K)
7    IF(K.EQ.LI)SLPHER=((DEP(LI+1)-HTL)*S(LI))/10.)*2+((DEP(LI+1)-
6    HTL)/10.)*2)**0.5
7    IF(K.EQ.LI) RL=SLPEN*ROUGH(LI)
7    IF(K.EQ.LIS)SLPLEN=((RL-(DEP(LIS)/10.))*S(LIS))**2+(RL-(DEP(LIS)/
6    10.))*2)**0.5
7    IF(K.EQ.LIS) RL=SLPLEN*ROUGH(LIS)
1 C    ADD UP SLOPE SECTION LENGTHS
7    TOTLEN=TOTLEN+SLPLEN
1 C    ADD UP (SLOPE LENGTH * ROUGHNESS FACTOR) VALUES
4    60 TOTRL=TOTRL+RL
1 C    COMPUTE FINAL ROUGHNESS FACTOR
7    FROUGH=TOTRL/TOTLEN
7    RETURN
7    END
  
```

RUFF0010
 RUFF0020
 RUFF0030
 RUFF0040
 RUFF0050
 RUFF0060
 RUFF0070
 RUFF0080
 RUFF0090
 RUFF0100
 RUFF0110
 RUFF0120
 RUFF0130
 RUFF0140
 RUFF0150
 RUFF0160
 RUFF0170
 RUFF0180
 RUFF0190
 RUFF0200
 RUFF0210
 RUFF0220
 RUFF0230
 RUFF0240
 RUFF0250
 RUFF0260
 RUFF0270
 RUFF0280
 RUFF0290
 RUFF0300
 RUFF0310
 RUFF0320
 RUFF0330
 RUFF0340
 RUFF0350
 RUFF0360
 RUFF0370
 RUFF0380
 RUFF0390
 RUFF0400
 RUFF0410
 RUFF0420
 RUFF0430
 RUFF0440
 RUFF0450
 RUFF0460
 RUFF0470
 RUFF0480
 RUFF0490
 RUFF0500
 RUFF0510
 RUFF0520
 RUFF0530
 RUFF0540

NAME	MODE	SIZE	DESCRIPTION	UNITS
R	R*4	1	CALCULATED CURVE	FEET
S	R*4	15	PROFILE SLOPES	FEET
T	R*4	1	PERIOD OF DEEP WATER WAVE	SEC
DB	I*2	27,13,7	VALUES OF R/H0 FUNCTION OF PDB,PDB1,PCH	*100
DL	I*2	15	HORIZONTAL DISTANCE FROM ORIGIN, INCREASING FROM LAND TO SEA	FEET
D1	I*2	1	FIRST INTERPOLATION VALUE OF R/H0	FEET
D2	I*2	1	SECOHD INTERPOLATION VALUE OF R/H0	FEET
D3	I*2	1	THIRD INTERPOLATION VALUE OF R/H0	FEET
H0	R*4	1	HEIGHT OF DEEP WATER WAVE	FEET
II	I*4	1	NO. OF SLOPE ON WHICH WAVE BREAKS	FEET
IQ	I*4	1	POINTER INTO ARRAYS OF ANSHERS AND FLAGS	FEET
NP	I*4	1	RUISER OF POINTS IN PROFILE	FEET
DCS	I*2	1	HYPOTHETICAL SLOPE	*100
DEP	I*2	15	PROFILE HEIGHT ASCENDING ORDER	FT*10
IFC	I*4	16	CONVERGENCE FLAGS	FT*10
IFD	I*4	1	DURRY FLAG	FEET
IFG	I*4	16	EXCEED TABLE FLAG	FEET
ISL	I*4	1	NO. OF SLOPE ON WHICH RUMUP LIMIT LIES	FEET
LIJ	I*4	16	TABLE OF ENDING SLOPES	FEET
PCH	I*2	5	VALUES OF D/H0 FOR ENTRY INTO DB	*10
PDB	I*2	27	VALUES OF SLOPE FOR ENTRY INTO DB*100	FEET
RAS	R*4	16	TABLE OF CALCULATED RUMUPS	FEET
SCC	I*2	12	SCALING FACTORS AS A FUNCTION OF SLOPE	FEET
SLO	I*2	12	SLOPE(TAN*10) FOR USE IN SCALING	FEET
WTL	I*2	1	WATER LEVEL	FT*10
DCHB	I*2	1	BREAKER DEPTH BY BREAKER HEIGHT RATIO	*1000
H0T2	I*2	1	H0/T**2	*1000
LISL	I*4	16	TABLE OF STARTING SLOPES	*1000
POBL	I*2	13	VALUES OF H0/T**2 FOR ENTRY INTO DB	*1000
C*****START OF SUBROUTINE				
IMPLICIT INTEGER*(D,P)				
INTEGER*2 H0T2,WTL,SLO(12),SCC(12),RS				
CORIION /OUT/ LISL(16),LIJ(16),RAS(16),IFG(16),IFC(16)				
CORIION /TD/ DEP(16),DL(16),S(16),HB(16),ROUGH(16),NP,WTL				
CORIION /SY/ PDB(27),POBL(13),PCH(8)				
CORIION /SZ/ DB(27,13,7)				
DATA SLO/ 10, 20, 40, 60, 80, 100, 140, 200, 300,				
1 500, 800, 1500/				
DATA SCC/ 1000, 1049, 1097, 1119, 1131, 1136, 1140, 1136, 1120,				
1 1089, 1052, 1000/				
C				

RUN00010
 RUN00020
 RUN00030
 RUN00040
 RUN00050
 RUN00060
 RUN00070
 RUN00080
 RUN00090
 RUN00100
 RUN00110
 RUN00120
 RUN00130
 RUN00140
 RUN00150
 RUN00160
 RUN00170
 RUN00180
 RUN00190
 RUN00200
 RUN00210
 RUN00220
 RUN00230
 RUN00240
 RUN00250
 RUN00260
 RUN00270
 RUN00280
 RUN00290
 RUN00300
 RUN00310
 RUN00320
 RUN00330
 RUN00340
 RUN00350
 RUN00360
 RUN00370
 RUN00380
 RUN00390
 RUN00400
 RUN00410
 RUN00420
 RUN00430
 RUN00440
 RUN00450
 RUN00460
 RUN00470
 RUN00480
 RUN00490
 RUN00500
 RUN00510
 RUN00520
 RUN00530
 RUN00540

```
-----1-----2-----3-----4-----5-----6-----7-----8
1 C SLOPE FUNCTIONS TO CALCULATE BREAKING DEPTH AS A FUNCTION OF
1 C BOTTOM SLOPE AND HAVE STEEPNESS (HEGSEL'S ANALYSIS,1972)
7 B(SLOPE)=1.0/(0.64*(1.0+EXP(-19.5/SLOPE)))
7 A(SLOPE)=1.36*(1.0-EXP(-19.0/SLOPE))
1 C IFG(IQ)=0
7 IFC(IQ)=0
1 C WAVE STEEPNESS
7 H0I2=H0*1000/(TWT)+.5
1 C
1 C COMPUTATION TO FIND BREAKING DEPTH AND SLOPE ON WHICH HAVE BREAKS
1 C
7 DO 10 IX=1,10
7 IF(HS(IX).EQ.0.0) GO TO 10
7 SLP=S(IX)
7 DCHB=10./(B(SLP)-A(SLP))*HB(IX)/(TWT))
7 DC=DCHB*HB(IX)
7 DTT=HTL-DC
7 ADC=DC/10.
7 IF(DTT.LT.DEP(IX+1)) GO TO 20
7 IF(DTT.GT.DEP(NP-1)) GO TO 40
4 10 CONTINUE
1 C HAVE CANNOT BREAK ON SLOPE BEFORE SLOPE IX
4 20 IF(DTT.GE.DEP(IX)) GO TO 30
7 DC=HTL-DEP(IX)
4 30 II=IX
7 JJ=IX+1
1 C COMPUTE DISTANCE FROM REFERENCE TO BREAKING POINT
7 CALL RINT(DEPII),DEP(JJ),DL(II),DL(JJ),DTT,DLE)
7 IFD=0
7 GO TO 50
4 40 II=NP-1
7 DLE=DL(II)+(HTL-DC-DEP(II))*S(II)/10.+5
1 C FIND FAMILY OF CURVES
4 50 R=0.
7 IF (H0.EQ.0.) GOTO 220
7 DH=DC/H0
7 LOOP UNTIL R SAME AS R1
7 CALL LOOK((PDB1,I3,H0T2,KK,LL,IFG(IQ))
7 IFD=0
7 DO 210 N=1,10
7 IFD=0
7 R10=R*10.
7 DIR=HTL+R10
7 NPP=NP-1
1 C FIND SLOPE THAT RUNUP LIMIT INTERSECTS
7 DO 60 IT=1,NPP
7 IF(DIR.LT. DEPI(IT)) GO TO 70
4 60 CONTINUE
7 GO TO 60
4 70 IAL=IT-1
7 ISL=IT
1 C COMPUTE DSL, THE DISTANCE FROM REFERENCE TO THE RUNUP LIMIT
7
```

```

START COL 1-----2-----3-----4-----5-----6-----7-----8
7 CALL RINT(DEP(IAL),DEP(IAL),DL(IAL),DL(IAL),DIR,DSL) RUN01090
1 C
1 C DETERMINE IF HAVE OVERTOPS SLOPE THAT WATER LEVEL INTERSECTS RUN01100
1 C IF NOT, THE COMPOSITE SLOPE METHOD IS NOT REQUIRED AND A ONE STEP RUN01110
1 C HAVE RUNUP CALCULATION IS PERFORMED RUN01120
7 IF(R.EQ.0) GO TO 100 RUN01130
7 GO TO 90 RUN01140
4 80 IAL=IP-1 RUN01150
1 COMPUTE DSL, THIS EQUATION COMPUTES DSL WHEN THE RUNUP LIMIT RUN01160
1 IS CH THE LAST LAHWARD SLOPE RUN01170
7 DSL=DL(IAL)*((R10+HTL-DEP(IAL))*S(IAL))/10+.5 RUN01180
4 DCS=1000*(DSL-DLE)/(DC*R10) RUN01190
7 GO TO 120 RUN01200
3 100 DS=-DEP(IAL)/H0 RUN01210
7 IF(DS.LE.25) GO TO 110 RUN01220
7 IF(S(IAL-1).GT.30) DS=DS*100 RUN01230
3 110 CALL LOCK(PCH,8,DS,IZ,R,IFD) RUN01240
7 DCS=S(IAL)*100 RUN01250
1 C RUN01260
3 120 IF(DCS.LI.1000) GO TO 140 RUN01270
7 IF(DCS.LI.3000.AND.IZ.GT.4) GO TO 140 RUN01280
1 C EXTRAPOLATE TO GET R RUN01290
7 XH=ALOG10(DCS/100.) RUN01300
7 IF(IZ.LE.4) GO TO 130 RUN01310
1 C RUN01320
1 C EXTRAPOLATE IN STOA TABLES 2,3,4 RUN01330
7 Y7K=DB(27,KK,IZ) RUN01340
7 Y7K=ALOG10(Y7K) RUN01350
7 Y4K=DB(24,KK,IZ) RUN01360
7 Y4K=ALOG10(Y4K) RUN01370
7 Y7L=DB(27,LL,IZ) RUN01380
7 Y7L=ALOG10(Y7L) RUN01390
7 Y4L=DB(24,LL,IZ) RUN01400
7 Y4L=ALOG10(Y4L) RUN01410
7 D1=10*((Y7K-Y4K)/0.477)*(XN-1.477)+Y7K) RUN01420
7 D2=10*((Y7L-Y4L)/0.477)*(XN-1.477)+Y7L) RUN01430
7 GO TO 150 RUN01440
1 C RUN01450
1 C EXTRAPOLATE IN STOA TABLES 6,9,10,11 RUN01460
3 130 Y7K=DB(24,KK,IZ) RUN01470
7 Y7K=ALOG10(Y7K) RUN01480
7 Y4K=DB(21,KK,IZ) RUN01490
7 Y4K=ALOG10(Y4K) RUN01500
7 Y7L=DB(24,LL,IZ) RUN01510
7 Y7L=ALOG10(Y7L) RUN01520
7 Y4L=DB(21,LL,IZ) RUN01530
7 Y4L=ALOG10(Y4L) RUN01540
7 D1=10*((Y7K-Y4K)/0.222)*(XN-1.000)+Y7K) RUN01550
7 D2=10*((Y7L-Y4L)/0.222)*(XN-1.000)+Y7L) RUN01560
7 GO TO 150 RUN01570
1 C RUN01580
1 C PULL DATA OUT OF TABLE DB RUN01590
3 140 CALL LOCK(PDB,27,DCS,IN,JJ,IFG(IQ)) RUN01600
1 C INTERPOLATE TO FIND R RUN01610
1 C RUN01620
  
```

-----1-----2-----3-----4-----5-----6-----7-----8

```

7 CALL LOGLOG(PDB(IH),PDB(JJ),PDB(KK),PDB(LL),PDB(MM),PDB(NN),PDB(OO),PDB(PQ),PDB(RR),PDB(SS),PDB(TT),PDB(UU),PDB(VV),PDB(WW),PDB(XX),PDB(YZ),DCS,D1)
7 CALL LOGLOG(PDB(IH),PDB(JJ),PDB(KK),PDB(LL),PDB(MM),PDB(NN),PDB(OO),PDB(PQ),PDB(RR),PDB(SS),PDB(TT),PDB(UU),PDB(VV),PDB(WW),PDB(XX),PDB(YZ),DCS,D1)
3 150 CALL LOGLOG(PDB1(MK),PDB1(LL),PDB1(DD),PDB1(EF),PDB1(GH),PDB1(IJ),PDB1(KL),PDB1(MN),PDB1(OP),PDB1(QR),PDB1(ST),PDB1(UV),PDB1(WX),PDB1(YZ),DCS,D2)
7 RI=H0*DS/100.
7 CALL RRUFF(RI,FROUGH,N)
7 RI=R1*FROUGH
7 IF((1.0-FROUGH).GT.0.01) GO TO 160
1 C_____SCALE EFFECT
7 IF((DCS.LT.1500).AND.(DCS.GT.10)) GO TO 160
7 RS=1000
7 GO TO 170
3 160 CALL LOOK(SLO,I2,DCS,IP,IPI,IFD)
1 C_____INTERPOLATE TO FIND SCALE EFFECT
7 CALL LOGLIH(SLO(IP),SLO(IPI),SCC(IP),SCC(IPI),DCS,RS)
3 170 RI=R1*RS/1000.
3 180 IF(RH.E.0.) GO TO 200
7 IF((R1*10).LT.DEP(ISLI)) GO TO 230
7 ..(DH.LE.25) GO TO 190
7 IF(S(I).GT.30) DH=DH*100
3 190 CALL LOOK(PCH,6,DH,IZ,K,IFD)
1 C_____CHECK FOR CONVERGENCE OF RUNUP
3 200 IF(A55IR-R1).LT.0.1) GOTO 220
7 LISL(IQ)=IAL
7 LI(IQ)=II
7 R=R1
7 RAS(IQ)=R
7 IQ=IQ+1
3 210 CONTINUE
7 IQ=IQ-1
7 IFC(IQ)=1
3 220 LISL(IQ)=IAL
7 LI(IQ)=II
7 RAS(IQ)=R
7 RETURN
3 230 LISL(IQ)=IAL
7 LI(IQ)=II
7 RAS(IQ)=R1
7 RETURN
7 END

```

RUN01630
 RUN01640
 RUN01650
 RUN01660
 RUN01670
 RUN01680
 RUN01690
 RUN01700
 RUN01710
 RUN01720
 RUN01730
 RUN01740
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 RUN01760
 RUN01770
 RUN01780
 RUN01790
 RUN01800
 RUN01810
 RUN01820
 RUN01830
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 RUN01860
 RUN01870
 RUN01880
 RUN01890
 RUN01900
 RUN01910
 RUN01920
 RUN01930
 RUN01940
 RUN01950
 RUN01960
 RUN01970
 RUN01980
 RUN01990
 RUN02000
 RUN02010

APPENDIX B

DETERMINATION OF SIGNIFICANT WAVE HEIGHT AND PERIOD

In the absence of a particular synthetic storm to be used to generate 100-year wave conditions, a reasonable choice of wind parameters needs to be made and a corresponding sea state computed.

- a. If the 100-year elevation can be linked to a particular storm (e.g., February 1978), then an appropriate set of wind data exists for modeling the sea state. Given wind speed, duration, and fetch, the significant wave parameters (height, period) can be computed. These waves will not be the most extreme which may be experienced but they will be waves which the coast will be subject to over the duration of the storm and represent the average state of the highest waves generated. This "average state" will be considered appropriate for computing wave runup.

Wave information can be obtained in a number of ways:

- i. Specific Storm Reports (e.g., "The Blizzard of 1978" - Its Effects on the Coastal Environments of Southeastern New England - Conference, Boston State College, Boston, Massachusetts, 1978).

- ii. Wave data recorded at Coast Guard stations and available from the National Weather Service.
 - iii. Generation of wave characteristics using actual or synthesized wind data and the techniques commonly adopted for wave prediction, e.g., Sverdrup-Munk-Bretschneider (SMB) as set forth in Shore Protection Manual.
- b. If the 100-year stillwater elevation cannot be related in approximate fashion to a specific storm, then, possibly, some general historical perspective can be gained by a knowledge of the characteristics of the historical storms which have impacted the area of interest. Using this knowledge, a synthetic storm could be pieced together which could subsequently be used to generate a wave field.
- c. The pertinent wind parameters can be independently adjusted to produce a given surge. However, this surge must be coupled with an astronomical tide to result in a total stillwater elevation equal to the 100-year elevation.

Approach (a) is the most desirable, followed by Approach (b). There are obvious difficulties with Approach (c) as the relative magnitude of the components of the 100-year elevation (surge, astronomical tide) is not known nor is it unique, i.e., numerous combinations are possible.

In this sample problem, Approach (a) was used to estimate the design deepwater wave parameters. Two gaging stations at Portsmouth and Portland represent the southern and northern limits of the study area.

Statistical analysis of tidal records for Portsmouth, New Hampshire, yields a 100-year elevation of 8.7 ft. The February 6-7, 1978, northeaster produced an elevation of 8.7 ft. At Portland, Maine, the corresponding numbers are 9.4 ft and 9.6 ft, respectively. This demonstrates that the February 1978 storm has the characteristics of the 100-year flood producing storm.

Wind data are readily available for the Boston area. Because of the size of the storm, these data are assumed valid for the southern Maine coastline as well. Wave hindcasting was done using the recently published Coastal Engineering Technical Notes (U.S. Coastal Engineering Research Center, March, 1981).

Since the southern Maine area has a large open ocean to the east, it is the magnitude, i.e., physical dimensions of the storm, which will determine the effective fetch.

The significant wave period and height predicted for the study area were 13.8 sec and 30 ft, respectively.

As collaboration for this computation the National Weather Service reported that during the February 1978 storm seas in open waters were 20-30 ft high. Referring to the wave forecasting charts, these wave heights yield a corresponding significant wave period from 12 to 14 sec. In addition, a 26-foot wave was reported at a drill rig near Hampton Harbor.

For the area under consideration, no distinction is made between the 10-year and 100-year wave conditions. This is because comparable storms producing comparable surge and wave magnitudes may not produce comparable stillwater elevations due to a difference in the astronomical tide component. Furthermore, the difference between the 10-year and 100-year stillwater elevations is small ($\cong 1$ ft).

In summary, the above computation and observations suggests a reasonable $H_s = 30$ ft and $T_s = 14$ sec.